



TRANSPORT AND TRAFFIC PLANNING ASSOCIATES

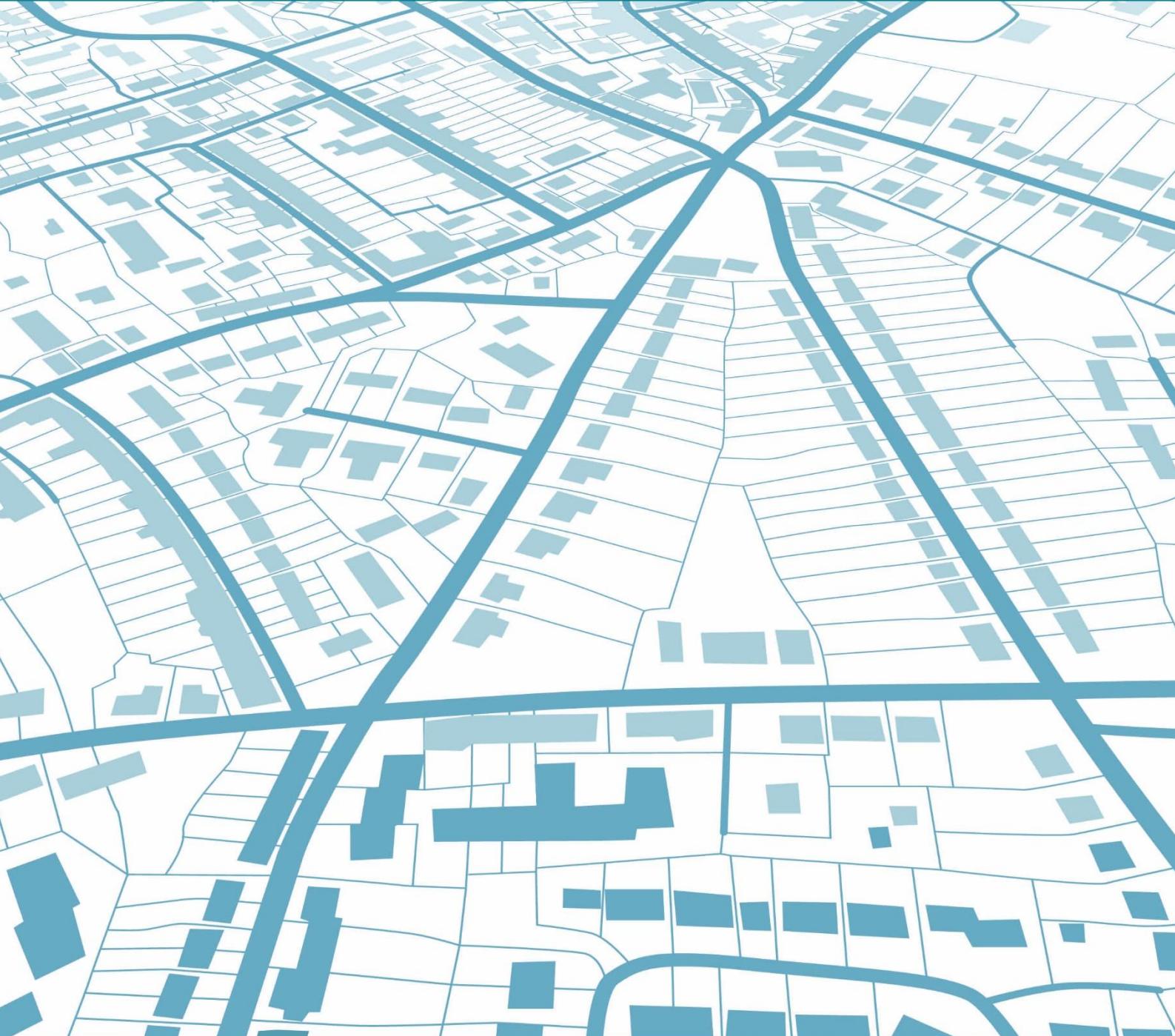
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Anglicare Milperra Village, Bullecourt Avenue, Milperra Proposed Residential Aged Care Development Traffic and Parking Assessment

Ref: 17161

Date: September 2019

Issue: B

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1.0 Introduction

This report has been prepared for Anglicare (a Social Housing Provider) to accompany a Development Application to Canterbury Bankstown Council for a proposed Seniors Living with self-contained dwellings (Independent Living Units (ILUs) and a Residential Aged Care Facility (RACF) on Bullecourt Avenue at Milperra (Figure 1).

The development site is located just to the east of Henry Lawson Drive and to the north of the M5 Motorway being a former southern “appendage” of the large Bankstown Golf Club landholding. The proposed development scheme comprises:

- * 81 ILU apartments
- * 107 RACF beds (33 for persons with dementia)
- * ancillary facilities
- * basement and surface parking

The purpose of this report is to:

- * describe the site, its context and the proposed development scheme
- * describe the road network serving the site and the prevailing traffic conditions
- * assess the adequacy of the proposed parking provision
- * assess the potential traffic implications
- * assess the suitability of the proposed vehicle access, internal circulation and servicing arrangements
- * assess the safety and suitability of the proposed pedestrian refuge across Bullecourt Avenue

2.0 Proposed Development

2.1 Site, Context and Existing Use

The site (Figure 2) is consolidation of Lots 161 and 272 of DP 752013 which occupies a generally square shaped area of some 27,600m² with frontages to Bullecourt Avenue and Bullecourt Lane. The lot, which was formerly part of the Bankstown Golf Course landholding, is largely vacant apart from a shed in the north-east corner was used in the past as a “practice fairway”.

The site is adjoined to the west by residential dwellings while other land uses in the vicinity of the property comprise:

- * the industrial uses extending to the east of Bullecourt Lane
- * the residential dwellings to the south
- * the golf course lands extending to the north
- * the large Bankstown Airport precinct further to the north
- * the UWS Bankstown Campus to the south-east

2.2 Proposed Development

It is proposed to clear the site and excavate parts to provide for basement car parking and level building platforms.

The proposed development will comprise:

RACF	ILUs (buildings)
107 beds (33 dementia)	7 x one-bed
Ancillary facilities	46 x two-bed
36 staff (maximum daytime)	28 x three-bed
Total 81 apartments	

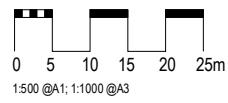
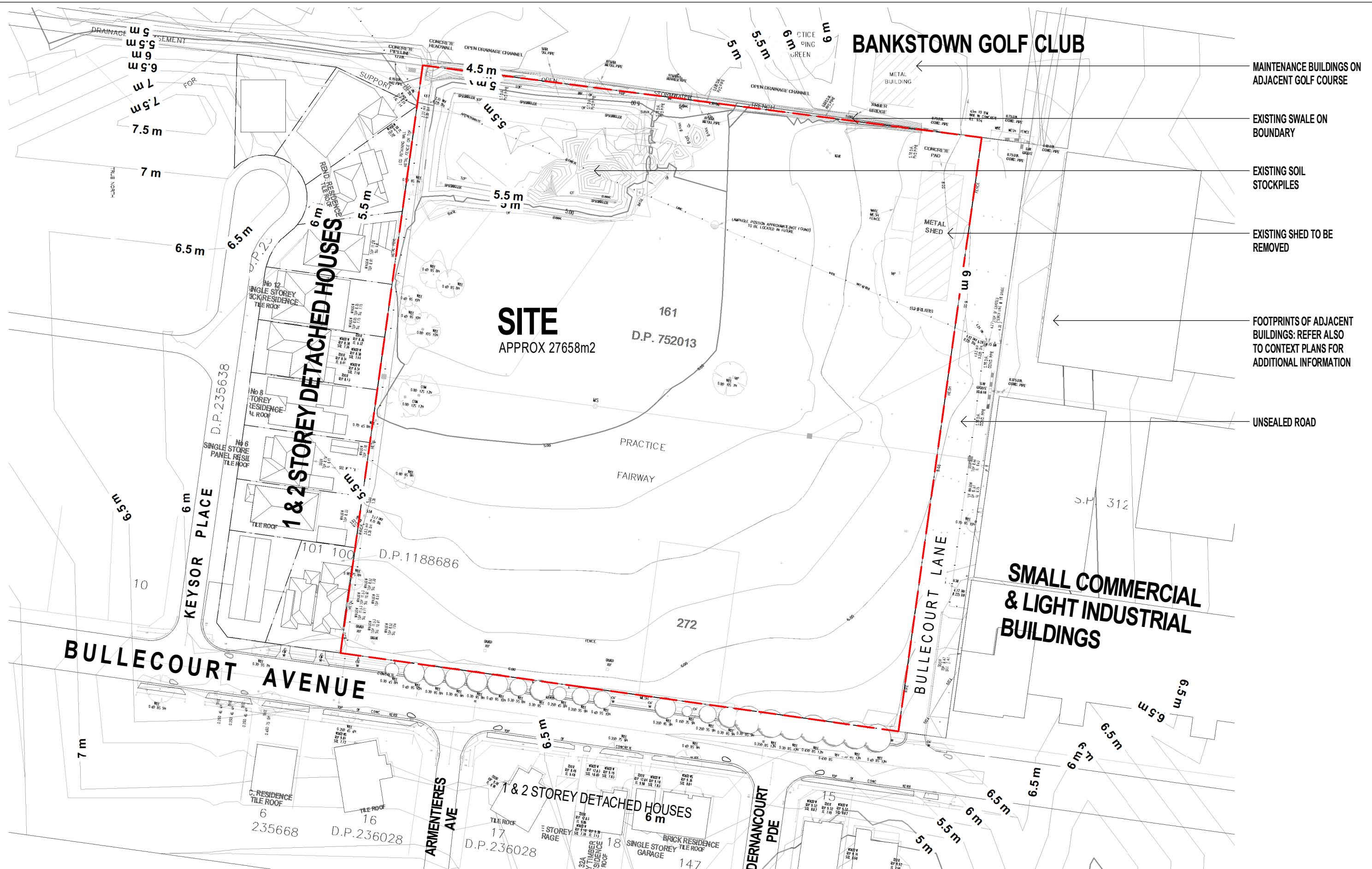


LEGEND



SITE

FIG 2



DA1	29.01.19	DA ISSUE	MI
REV	DATE	DETAILS	INITIALS

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CLIENT:
ANGLICARE (ACS),
SYDNEY

PROJECT:
MILPERRA VILLAGE

DRAWING TITLE:
EXISTING SITE PLAN

DATE:
JAN 29, 2019

DRAWN:
1:500@A1

SCALE:
1:500@A1

CHECKED:
973_DA2_SITE_Central.rvt

PROJ.: MILPERRA

DRAWING: DA.S.05

REVISION: DA1

A total of 138 parking spaces (including 11 accessible and 1 ambulance spaces) will be provided in basement and at-grade areas with driveways located on the Bullecourt Avenue and Bullecourt Lane frontages as well as a porte cochere access from Bullecourt Lane.

Details of the proposed development scheme are provided on the development plans prepared by SitePlus, which accompany the Development Application and are reproduced in part in Appendix A.

3.0 Road Network and Traffic Conditions

3.1 Road Network

The road network serving the site (Figure 3) comprises:

- * *M5 Motorway* – a State Road and arterial route connecting between Mascot and Prestons with access provided at the Henry Lawson Drive interchange
- * *Milperra Road* – a State Road and arterial route
- * *Henry Lawson Drive* – a State Road and sub-arterial route
- * *Bullecourt Avenue, Horsley Drive and Beaconsfield Road* – a Regional Road and important collector route

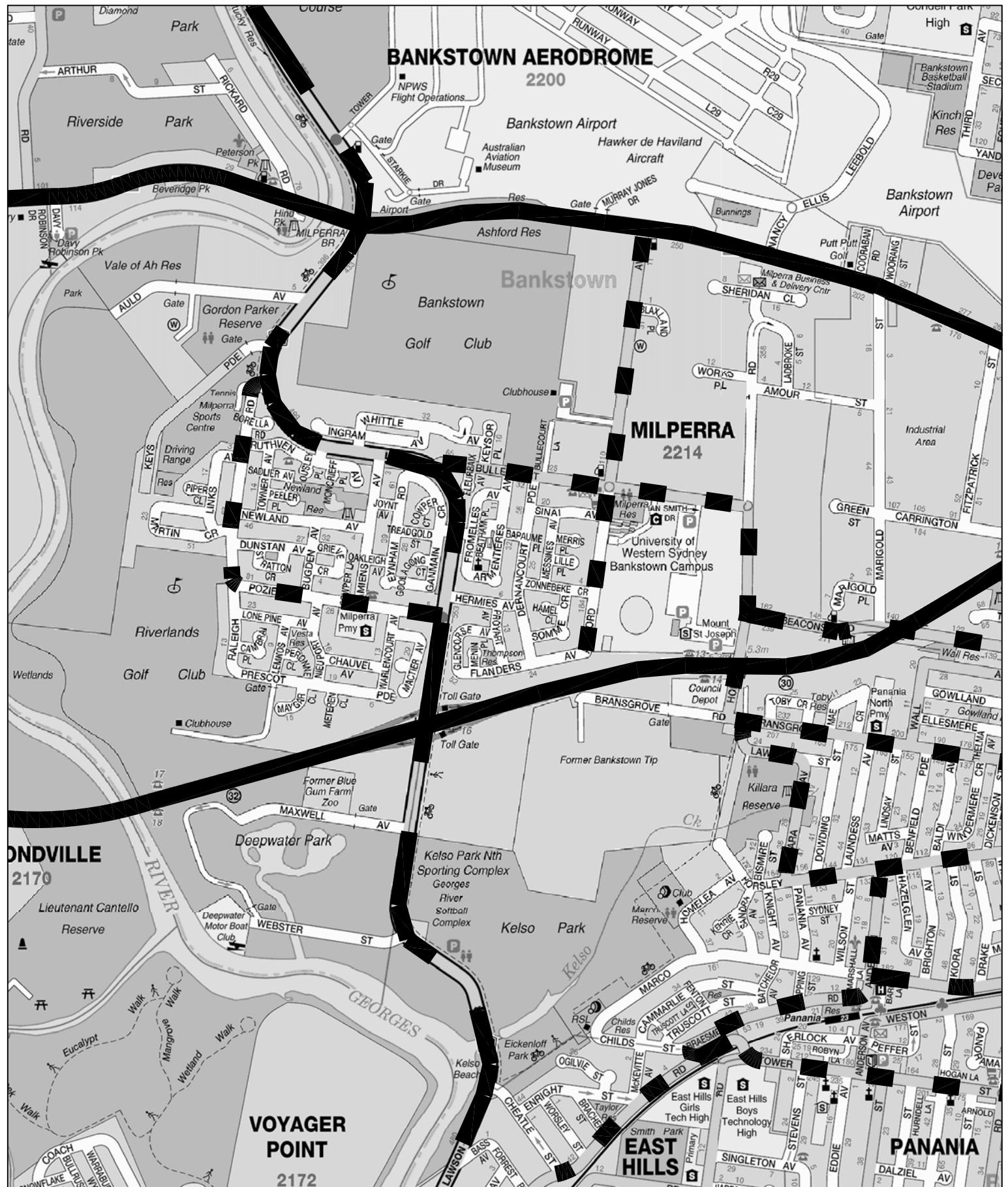
The M5 Motorway presents a “barrier” to the road network although overbridges are provided on Henry Lawson Drive, Horsley Road and Beaconsfield Road.

Bullecourt Avenue is relatively straight and level with one traffic lane and kerbside parking in each direction while Bullecourt Lane is an unsealed cul-de-sac.

3.2 Traffic Controls

The existing traffic controls (Figure 4) on the road system in the vicinity of the site include:

- * the traffic signal controlled intersections along Henry Lawson Drive including the Bullecourt Avenue and the M5 Ramp intersections
- * the roundabouts at the Horsley Road /Bullecourt Avenue and Ashford Avenue /Bullecourt Avenue intersections



LEGEND

ARTERIAL

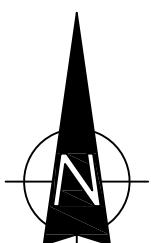
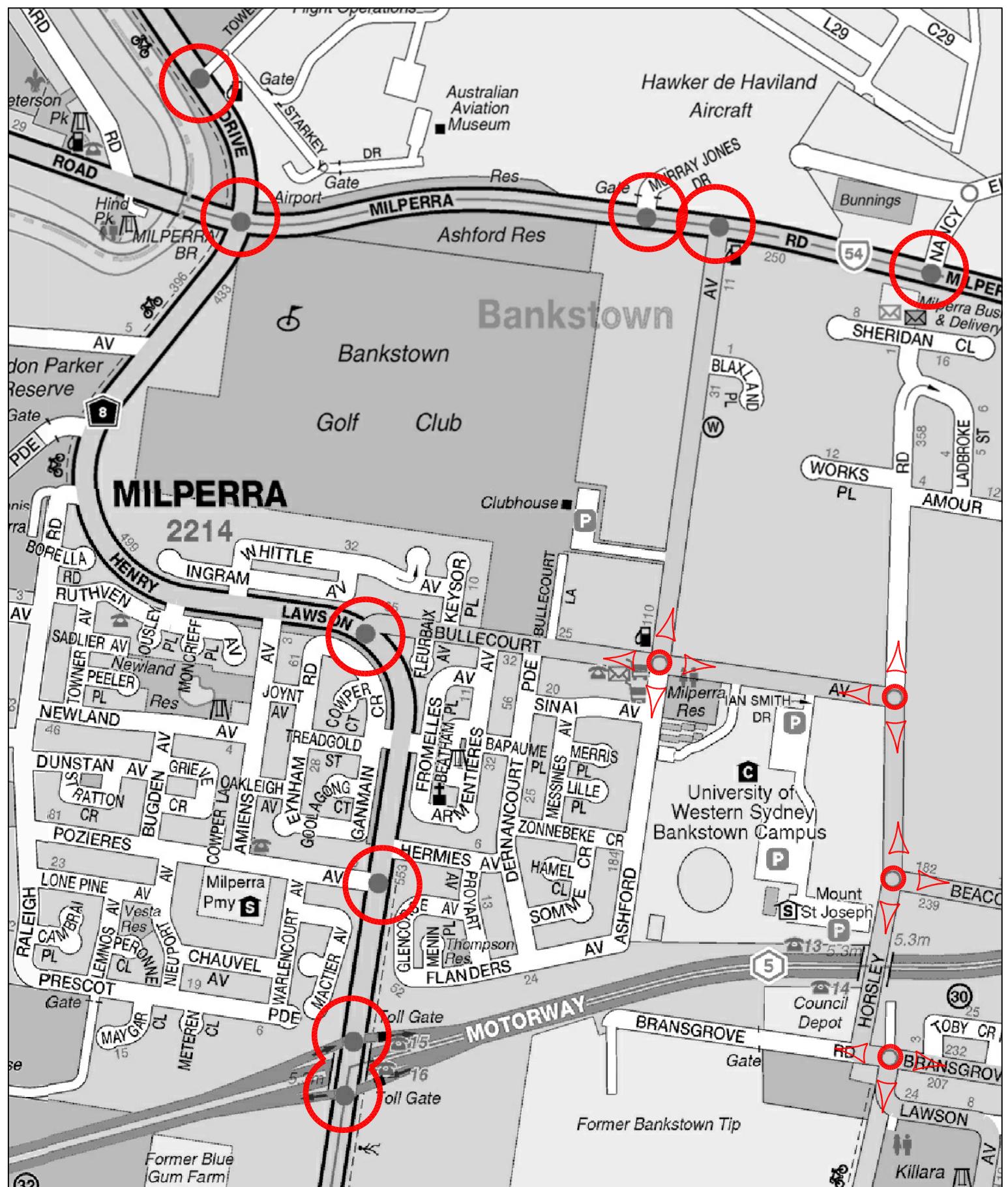
SUB-ARTERIAL

COLLECTOR



ROAD NETWORK

FIG 3



TRAFFIC CONTROLS

FIG 4

- * the traffic signal controlled intersections along Milperra Road including the Henry Lawson Drive intersection
- * the 50 kmph speed limit on the local road and collector system
- * the bus stops along Bullecourt Avenue

3.3 Traffic Conditions

An indication of the traffic conditions in the vicinity of the site is provided by data published by the RMS and traffic surveys undertaken as part of this assessment. The data¹ published by the RMS is expressed in terms of Annual Average Daily Traffic (AADT) and the latest available details are provided in the following:

	AADT
Henry Lawson Drive	23,900

Traffic surveys have been undertaken the AM and PM peak periods at the Bullecourt Avenue /Bullecourt Lane and Dernancourt Parade intersection complex. The results of those surveys are provided in Appendix B and summarised in the following:

		AM	PM
Bullecourt Avenue	Eastbound	689	297
	Left Turn	-	1
	Westbound	269	550
	Right Turn	-	-
Bullecourt Lane	Right Turn	-	1
	Left Turn	-	-
Bullecourt Avenue	Eastbound	687	292
	Right Turn	15	17
	Westbound	258	515
	Left Turn	11	35
Dernancourt Parade	Right Turn	2	6
	Left Turn	18	15

¹ *Traffic Volumes for Roads and Maritime Services*

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An annual 2% compound growth rate has been adopted to adjust the surveyed 2017 traffic volumes to Year 2019 volumes. This considers both traditional background growth and recognises other sites in Milperra and adjacent suburbs that are under construction or in planning.

Assessment of the intersection using Traffic Modelling program SIDRA indicates a satisfactory operation. Details of the model output are provided in Appendix C and summarised as follows while a guide to interpreting SIDRA results is reproduced overleaf:

Intersection	AM			PM		
	AVD	LOS	DS	AVD	LOS	DS
Bullecourt Ave/ Dernancourt Pde	14.1s	B	0.01	11.7s	B	0.02
Bullecourt Ave/ Bullecourt Ln	14.0s	B	0.01	11.7s	B	0.01

It is apparent that the operational performance of the Bullecourt Avenue/Dernancourt Parade and Bullecourt Avenue/Bullecourt Lane intersections is quite satisfactory due to the regular lengthy gaps in the Bullecourt Avenue traffic flows.

A review of the traffic survey data shows that the peak hours along Bullecourt Avenue in the vicinity of the site occur between 8.00am and 9.00am and between 3.30pm and 4.30pm during the AM and PM peak periods respectively.

An additional traffic survey was completed on Thursday, 29 November 2018 to determine the vehicle queue along Bullecourt Avenue in the eastbound direction towards Western Sydney University during the AM peak hour and vehicle queue along Bullecourt Avenue in the westbound direction towards Henry Lawson Drive . The longest vehicle queue waiting to enter the roundabout of Bullecourt Avenue/Ashford Avenue occurred at 8.35am and the vehicle queue lasted no more than a minute (see figure below). This queue only extended 140m from the roundabout, 60m (10 vehicles) to the east of the site access at Bullecourt Lane.

Criteria for Interpreting Results of SIDRA Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good	Good
'B'	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
'C'	Satisfactory	Satisfactory but accident study required
'D'	Operating near capacity	Near capacity and Accident Study required
'E'	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode	At capacity and requires other control mode
'F'	Unsatisfactory and requires additional capacity	Unsatisfactory and requires other control mode

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below, which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabouts	Give Way and Stop Signs
A	Less than 14	Good operation	Good operation
B	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
C	29 to 42	Satisfactory	Satisfactory but accident study required
D	43 to 56	Operating near capacity	Near capacity and accident study required
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode	At capacity and requires other control mode

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by **traffic signals**¹ both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a **roundabout or GIVE WAY or STOP signs**, satisfactory intersection operation is indicated by a DS of 0.8 or less.

¹ the values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs



As such, ingressing vehicles from westbound Bullecourt Avenue into the site and egressing vehicles onto westbound Bullecourt Avenue will not be affected by this queue during the AM peak hour.



In the PM peak hour, while the westbound vehicle queue travelling along Bullecourt Avenue extends through the western site access due to the signalized intersection of

Bullecourt Avenue/Henry Lawson Drive, the queue occurs twice an hour and lasted no more than a minute (see figure below), allowing more than sufficient gaps for vehicles to exit via the western access.



3.4 Transport Services

The bus services which operate along Bullecourt Avenue comprise:

- Route M90 which connects between Liverpool and Bankstown railway station
- Route 55 which connects between Milperra and Padstow railway station
- Route 922 which connects between East Hills and Bankstown railway station
- Route 962 which connects between East Hills and Miranda

The site is therefore conveniently located with bus services providing connection to the Metropolitan transport network.

4.0 Parking

The minimum parking provision relevant to the proposed Social Housing Provider development is specified in the SEPP (Housing for Seniors or People with Disability) as follows:

ILU Apartments	RACF
1 space per 5 ILUs	1 space per 10 beds (or 1 space per 15 beds if the facility provides care only for persons with dementia)
	1 space per 2 staff
	Ambulance space

Given the location of the site and ease of access to convenient public transport services, the Roads and Maritime Guide has also been referenced to better understand the parking requirements for visitors. The parking requirement is provided as follows:

ILU Apartments	RMS Minimum Parking Rate
Self-contained units (subsidised development)	1 visitor space per 10 ILUs

Application of the above criteria would indicate the following provision:

ILU Apartments		RACF	
81 ILUs	<ul style="list-style-type: none"> - 17 residents/ tenants spaces - 8 visitor spaces 	74 beds	<ul style="list-style-type: none"> - 7.4 (8) spaces
Total	25 spaces	33 dementia beds	<ul style="list-style-type: none"> - 2.2 (3) spaces
		36 staff	<ul style="list-style-type: none"> - 18 spaces
			1 ambulance space
		Total	<ul style="list-style-type: none"> - 30 spaces
			(including 1 ambulance space)

It is proposed to provide:

ILU residents/tenants	-	82 spaces (6 accessible)
ILU visitors	-	22 spaces (2 accessible)
Subtotal for ILU	-	104 spaces (8 accessible)
RACF staff	-	25 spaces (2 accessible)
RACF visitors	-	8 spaces (1 accessible)
RACF	-	1 ambulance space
Subtotal for RACF	-	34 spaces (3 accessible and 1 ambulance)
Total	-	138 spaces (11 accessible and 1 ambulance)

This parking provision exceeds the minimum required providing for miscellaneous visitors (doctors, service personnel, etc.) and will act to ensure that the development does not need to rely on on-street parking. There will also be separate provision for delivery/service vehicles (up to a 12.5m heavy rigid vehicle (HRV)), wash bay and mini bus.

5.0 Traffic

ILUs

The Roads and Maritime Guide recommends a rate of 0.4 vehicle trips (residents/tenants/visitors) per occupied dwelling during the weekday peak period. Application of these factors to the proposed development would indicate the following:

	AM	PM
81 ILUs	33 vtph	33 vtph

The proposed ILUs will generate up to 33 vehicle trips during the peak hours.

RACF

It is considered appropriate to adopt a first principles assessment to determine the future number of vehicle trips generated by the proposed RACF use. The proposed staff spaces are anticipated to generate in the order of 0.5 peak hour vehicle trips per space. This rate is reflective of the nature of shift work employment and the varying start times and finish times for staff.

The proposed visitor spaces are not expected to generate vehicle trips during the peak periods. As such, it is recommended that a rate of 0.15 peak hour vehicle trips per visitor space be adopted.

Based on the above, the proposed RACF could potentially generate up to 15 vehicle trips (13 staff trips and 2 visitor trips) in any weekday peak hours.

Summary

Application of these factors to the proposed development would indicate the following:

	AM Peak	PM Peak
81 ILUs	33 vtph	33 vtph
RACF 107 Beds	15 vtph	15 vtph
Total	48 vtph	48 vtph

The projected access movements are as follows:

	AM Peak	PM Peak
Bullecourt Avenue Access		
RT IN	4	3
LT IN	2	1
RT OUT	1	2
LT OUT	3	4
Bullecourt Lane Access		
RT IN	10	17
LT IN	3	8
RT OUT	7	3
LT OUT	18	10

An assessment of the intersection operation 10 years post development has also been completed to safeguard the intersection layout and operation. The additional traffic resultant from the proposal has been distributed onto the 10 years post development traffic flows and reassessed using SIDRA. The outcome of the assessment is summarised in the following and provided in Appendix C:

Intersection	AM			PM		
	AVD	LOS	DS	AVD	LOS	DS
Bullecourt Ave/	15.0	B	0.02	13.9s	B	0.30
Dernancourt Pde						
Bullecourt Ave/	14.8s	B	0.10	13.9s	B	0.03
Bullecourt Ln						

The results of the assessment indicates that the intersections would continue to operate well with minimal delays and queuing for all approaches. The Level of Service for the intersections will be maintained, noting the very minor increase in average vehicle delay of 2.2s. In the light of the above, the projected development traffic is not expected to have perceptible implications on the intersections' operation with the future intersections' operational performance consistent with the existing condition. As discussed, there are regular lengthy gaps in the Bullecourt Avenue traffic flows. It is

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also apparent that there will be no adverse traffic implications at either of the access points on Bullecourt Avenue and there will be sufficient capacity at the nominated intersections well into the future.

6.0 Access, Internal Circulation, Servicing and Pedestrian Facilities

6.1 Access

The proposed vehicle access arrangements for the development will comprise:

- * a 6m wide combined ingress/egress driveway on the Bullecourt Avenue frontage at the western site boundary for Buildings C and D
- * a 6.8m wide combined ingress/egress driveway on the Bullecourt Lane located towards the northern site boundary for Buildings A and B basement parking and at-grade visitor parking
- * a 6m wide combined ingress/egress driveway on the Bullecourt Lane located towards the mid-eastern site boundary for the at-grade visitor parking and egress for the adjacent at-grade visitor parking and porte cochere of the RACF
- * a separate 6.2m wide access (one-way) on the Bullecourt Lane frontage for the RACF main entrance and at-grade visitor spaces
- * a 10m wide combined ingress/egress driveway in the southern part of the Bullecourt Lane frontage for RACF service/delivery vehicles and RACF basement parking

The proposed driveways will be located on straight and relatively level sections of roadway where excellent sight distances will be available. The design of the driveways will comply with the requirements of AS2890.1 & 2 and they will accommodate all vehicles requiring to use the driveways.

6.2 Internal Circulation

The ramps and access aisles provided in the basement and at-grade carparks will comply with the requirements of AS2890.1 & 6. Adequate provision will be available for

cars to manoeuvre into and out of the parking bays, which also comply with Council's design the criteria.

6.3 Servicing

Provision is made for a service, delivery and refuse collection vehicles in the loading area at the RACF Building as illustrated on the swept path diagrams of an HRV provided in Appendix D. Small service vehicles (e.g. service personnel) will be able to utilise the available visitor parking spaces while access and parking provision is made for a 7.73m 25-seater Fuso mini bus for transporting village residents.

6.4 Pedestrian Facilities

Pedestrian access to the site is proposed via a continuous footpaths along the northern side of Bullecourt Avenue and the western side of Bullecourt Lane. The footpaths along the Bullecourt Avenue frontage provides pedestrian linkages from Bullecourt Avenue to the internal footpaths to Buildings A, B, C and D as well as the RACF building within the site and connects to Bullecourt Lane, ensuring good permeability with the on-site facilities.

There are currently limited connection and crossing opportunities along Bullecourt Avenue. As such, it is recommended that a pedestrian refuge island be provided along Bullecourt Avenue between Dernancourt Parade and Bullecourt Avenue. The pedestrian refuge island location has been generally designed in accordance with RMS standard T000962.

The pedestrian refuge island will assist pedestrians to cross Bullecourt Avenue safely by providing a space to wait for a gap in the traffic to complete the crossing in two stages. The proposed crossing facilitate convenient and safe pedestrian access to/ from the site from/to the nearest bus stop to the site located just to the southeast of the intersection of Bullecourt Avenue/Bullecourt Lane, as well as to the retail and commercial areas to the east via the existing footpaths on Bullecourt Avenue. In summary, the location of the pedestrian refuge island is appropriate for the use of seniors crossing Bullecourt Avenue.

7.0 Conclusion

The traffic and parking assessment undertaken for the proposed Anglicare ILU apartment and RACF development on a site in Bullecourt Avenue at Milperra has concluded that:

- ❖ the traffic generation of the proposed development will not present any adverse traffic implications
- ❖ the proposed parking provision will be adequate of the proposed development and will ensure that no overflow parking will occur within the surrounding road network
- ❖ the proposed access, internal circulation and parking arrangements will be appropriate and will accord with AS2890 series.
- ❖ the proposed pedestrian refuge for seniors crossing Bullecourt Avenue will be safe and appropriate

Appendix A

Development Plans

PROPOSED SENIORS LIVING DEVELOPMENT

MILPERRA, ANGLICARE

CIVIL ENGINEERING DEVELOPMENT APPROVAL PLANS

LEGEND PROPOSED

--- - - -	STORMWATER PIPE
██████████	GRADED PIT
× × × × ×	JUNCTION PIT
███	KERB INLET PIT
██	SEDIMENT FENCE
ST	SEDIMENT TRAP
\\\\\\\\\\\\\\\\	TEMPORARY CONSTRUCTION EXIT

LEGEND EXISTING

— — — —	STORMWATER LINE
□	STORMWATER PIT
— — — —	WATER LINE
— — — —	SEWER LINE
— — — —	TELSTRA LINE
+ 10.00	EXISTING SPOT LEVEL
— — — —	EXISTING CONTOURS
— — — —	GAS LINE
— — — —	ELECTRICAL LINE



Sheet List Table

Sheet Number	Sheet Title
01	TITLE PAGE
02	CIVIL WORKS PLAN
03	BULK EARTHWORKS PLAN
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05	TYPICAL ROAD CROSS SECTIONS 2
06	BUILDING C BASEMENT ENTRY CH0 TO CH32.727 LONGSECTION
07	BULLECOURT LANE CH0 TO CH100 LONGSECTION
08	BULLECOURT LANE CH100 TO CH187 LONGSECTION
09	ROAD NO.1 CH0 TO CH33.640 LONGSECTION
10	ROAD NO. 2 CH0 TO CH94.874 LONGSECTION
11	ROAD NO.3 CH0 TO CH39.867 LONGSECTIONS
12	ROAD NO. 4 CH0 TO CH4.0 LONGSECTION
13	ROAD NO. 5 CH0 TO CH94.822 LONGSECTION
14	PIT CATCHMENT PLAN
15	DRAINAGE CALCULATIONS
16	PIT SCHEDULE
17	DRAINAGE LONGSECTIONS 1
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21	DRAINAGE LONGSECTIONS 5
22	OVERLAND FLOW CHANNEL DETAILS
23	SOIL AND WATER MANAGEMENT PLAN
24	SOIL AND WATER MANAGEMENT DETAIL
25	B8.8m ENTRY SWEEP PATH 1
26	B8.8m EXIT SWEEP PATH 2
27	7.73m MINI BUS ENTRY AND EXIT SWEEP PATH
28	B99 ENTRY SWEEP PATH 1
29	B99 EXIT SWEEP PATH

GENERAL NOTES

- G1. ALL WORK TO BE CARRIED OUT IN ACCORDANCE WITH BANKSTOWN COUNCIL'S STANDARD SPECIFICATIONS AND TO THE REQUIREMENTS OF THE PCA.
- G2. INSPECTION BY LOCAL PLANNING OR CIVIL ENGINEER SHALL BE CARRIED OUT AT THE FOLLOWING STAGES:
 - (a) PRIOR TO INSTALLATION OF EROSION AND SEDIMENT CONTROL STRUCTURES
 - (b) PRIOR TO BACKFILLING PIPELINE, CURBOS, DRAINS, TRENCH BEDDING AND DAMS
 - (c) PRIOR TO CASTINGS OF PIPES AND OTHER CONCRETE STRUCTURES, INCLUDING KERB AND GUTTER
 - (d) PROOF ROLLER TEST OF SUBGRADE AND SUB-BASE
 - (e) FORMWORKS PRIOR TO POURING CONCRETE IN PARKING AREA FOR FOOTPATH CROSSING AND OTHER ASSOCIATED WORK
 - (f) PRIOR TO BACKFILLING PUBLIC UTILITY CROSSINGS IN ROAD RESERVES
 - (g) PRIOR TO PLACEMENT OF ASPHALTIC CONCRETE
 - (h) PRIOR TO PLACEMENT OF ASPHALTIC CONCRETE
 - (i) FINAL INSPECTION AFTER ALL WORKS ARE COMPLETED AND 'WORKS AS EXECUTED' PLANS HAVE BEEN SUBMITTED TO COUNCIL.
- G3. MAKE SMOOTH JUNCTIONS WITH EXISTING WORKS.
- G4. NO WORK TO BE CARRIED OUT ON COUNCIL PROPERTY OR ADJOINING PROPERTIES WITHOUT THE WRITTEN PERMISSION FROM THE OWNER.
- G5. VEHICULAR ACCESS AND ALL SERVICES TO BE MAINTAINED AT ALL TIMES TO ADJOINING PROPERTIES AFFECTED BY CONSTRUCTION.
- G6. ALL RUBBISH, BUILDINGS, SHEDS AND FENCES TO BE REMOVED TO SATISFACTION OF PCA AT COMPLETION OF WORKS.
- G7. A TRAFFIC CONTROL PLAN IS TO BE SUBMITTED TO COUNCIL WITH A SECTION 138 APPLICATION PRIOR TO COMMENCEMENT OF WORKS.
- G8. ALL FILL AREAS ARE TO BE INSPECTED BY A GEOTECHNICAL ENGINEER PRIOR TO STRIPPING AND ANY RECOMMENDATIONS REGARDING TREATMENT OF SALINE AFFECTED AREAS ARE TO BE IMPLEMENTED.
- G9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE VERIFICATION OF THE LOCATION OF ANY EXISTING SERVICES AFFECTING THE WORKS AREA, ANY DAMAGED SERVICES SHALL BE REPAIRED AT THE CONTRACTOR'S COST.

EARTHWORKS NOTES

- E1. EARTHWORKS TO BE CARRIED OUT TO THE SATISFACTION OF THE PCA AND GEOTECHNICAL ENGINEER. UNSOUND MATERIALS ARE TO BE REMOVED FROM ROADS AND BUILDING PADFS PRIOR TO FILLING. THE CONTRACTOR IS TO ARRANGE AND MAKE AVAILABLE COMPACTION CERTIFICATES WHERE REQUIRED.
- E2. WHERE THE SLOPE OF THE NATURAL SURFACE EXCEEDS ONE IN FOUR (1:4), BENCHES ARE TO BE CUT TO PREVENT SLIPPING OF THE PLACED FILL MATERIAL AS REQUIRED BY THE PCA AND GEOTECHNICAL ENGINEER.
- E3. ALL BATTERS ARE TO BE SCARIFIED TO ASSIST WITH ADHESION OF TOP SOIL TO BATTER FACE.
- E4. PROVIDE MINIMUM 50mm AND MAXIMUM 300mm TOPSOIL WITH GRASS SEEDING ON FOOTPATHS, FILLED AREAS AND ALL OTHER AREAS TURNED OVER DURING CONSTRUCTION.
- E5. THE CONTROL TESTING OF EARTHWORKS SHALL BE IN ACCORDANCE WITH THE GUIDELINES IN AUSTRALIAN STANDARD 3798 - 1996, WHERE IT IS PROPOSED TO USE TEST METHOD AS1789.6.1 OR AS1789.6.2 TO DETERMINE THE FIELD DENSITY, A SAND REPLACEMENT METHOD SHALL BE USED TO CONFIRM THE RESULTS AS DIRECTED BY PCA. THE GEOTECHNICAL TESTING AUTHORITY SHALL HAVE A LEVEL 1 RESPONSIBILITY FOR ALL FILLING AS DEFINED IN APPENDIX B AS 3798-1996, AND AT THE END OF THE WORKS SHALL CONFIRM THE EARTHWORKS COMPLY WITH THE REQUIREMENTS OF THE SPECIFICATION AND DRAWINGS.
- E6. THE CONTRACTOR SHALL CONTROL SEDIMENTATION, EROSION AND POLLUTION DURING CONSTRUCTION IN ACCORDANCE WITH THE REQUIREMENTS OF THE SOIL CONSERVATION SERVICES AND WATER RESOURCES OF NSW.

ROADWORKS NOTES

- R1. SUBGRADE, SUB-BASE, AND BASE TO BE COMPACTED IN ACCORDANCE WITH SITEPLUS CIVIL DWG CO4 AND GEO-TECHNICAL RECOMMENDATION SPECIFICATION.
- R2. SUBSOIL DRAINS TO BE PROVIDED ON BOTH SIDES OF ROADS (EXCEPT WHERE THERE IS STORM WATER DRAINAGE).
- R3. SERVICE CONDUITS TO BE PLACED AS DIRECTED BY INTEGRAL ENERGY, TELSTRA AND AS REQUIRED BY THE SYDNEY WATER.
- R4. PROPOSED SERVICES CROSSING EXISTING ROADS SHALL BE THRUST BORED UNDER THE ROAD SO AS NOT TO DAMAGE EXISTING SURFACE.
- R5. ALL ROADS ARE TO BE TEMPORARILY SEALED WITH 20mm GAP GRADED AC14, THE FINAL AC IS TO BE PLACED FOLLOWING CONSTRUCTION OF ADJOINING BUILDINGS.
- R6. SIGNPOSTING AND LINEMARKING TO CONFORM WITH AS1742.2 RAISED RETRO-REFLECTIVE PAVEMENT MARKERS TO CONFORM WITH AS1906. ALL APRONS AND KERB FACE ON CENTRAL ISLANDS OF ROUNDABOUTS AND ALL OTHER ISLANDS SHOULD BE DELINEATED BY REFLECTIVE WHITE MARKING.
- R7. ALL LEVELS ARE TO BE SET OUT FROM ESTABLISHED STATE SURVEY MARKS.

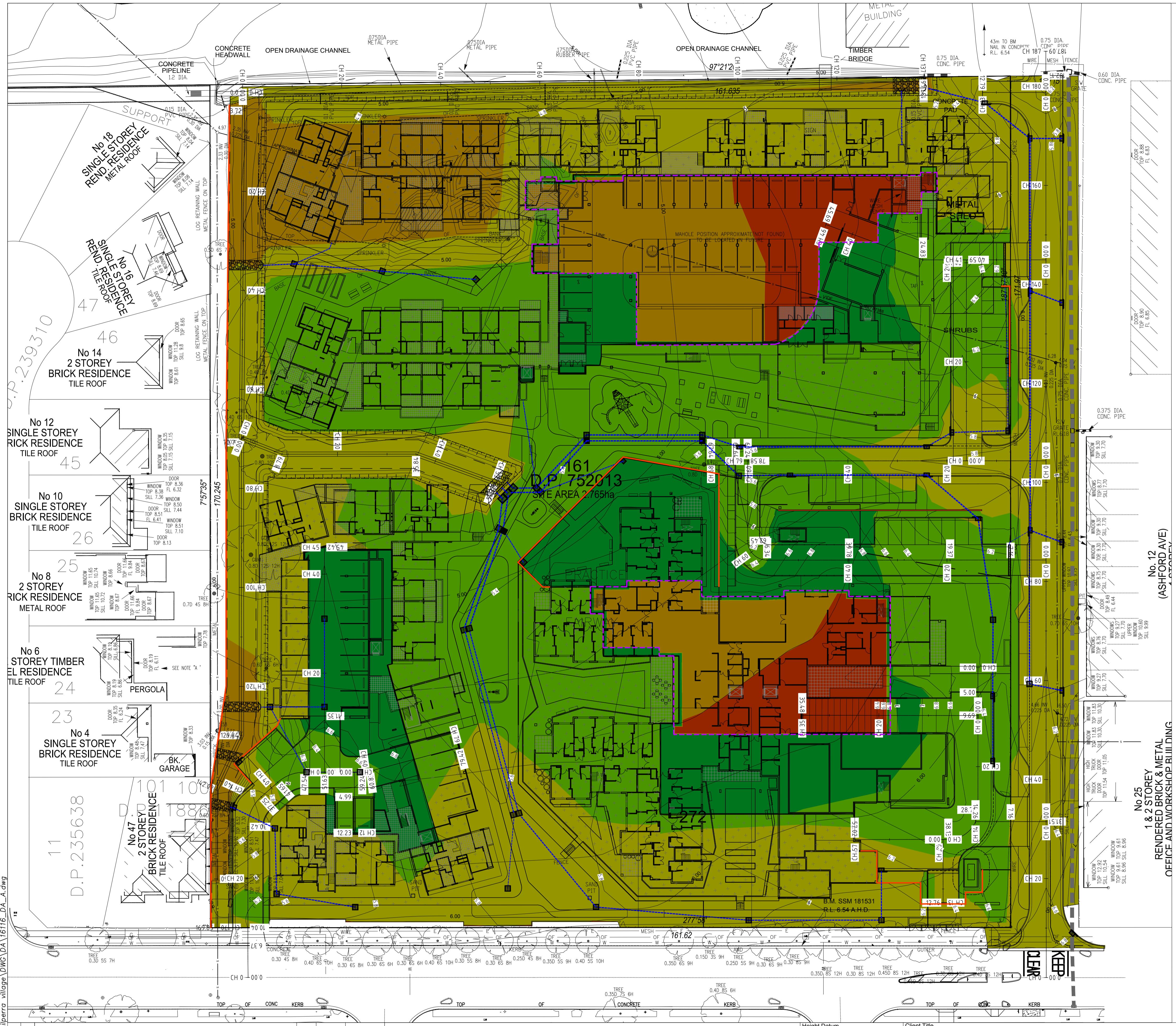
STORMWATER NOTES

- S1. ALL PIPES TO BE SPIGOT AND SOCKET, RUBBER RING JOINTED. ALL PIPES IN UNDER ROADS (OTHER THAN ROOF WATER AND SUBSOIL) ARE TO BE STEEL, REINFORCED CONCRETE PIPES.
- S2. ALL LONGITUDINAL PIPELINES IN ROADS MUST BE LOCATED UNDER KERB AND GUTTER AND BE BACKFILLED WITH 7mm AGGREGATE WHERE 10-15% OF FINES IS ALLOWABLE UNLESS OTHERWISE INDICATED ON PLANS.
- S3. DRAINAGE LINES MUST BE BACKFILLED WITH CLEAN SHARP SAND IN TRAFFICABLE AREAS.
- S4. ALL GULLY PITS TO COUNCIL'S STANDARD AND LINTELS CENTRALLY PLACED AT SAG PITS.
- S5. ALL PITS MUST BE BENCHED AND STREAMLINED. PROVIDE RF7 REINFORCEMENT AND STEP IRONS IN ALL PITS OVER 1.2M DEEP.
- S6. CONCRETE TO HAVE MINIMUM COMPRESSIVE STRENGTH OF 25 MPa AT 28 DAYS UNLESS SPECIFIED OTHERWISE BY COUNCIL ENGINEER.
- S7. ALL DRAINAGE MUST HAVE A MINIMUM COVER OF 300mm TO THE TOP OF PIPE UNLESS OTHERWISE APPROVED BY THE P.C.A.
- S8. CATCH DRAINS MUST BE CONSTRUCTED AS PER SWP.
- S9. ONE HUNDRED (100) YEAR OVERLAND FLOW PATHS MUST BE FORMED AND SHOWN ON WORK AS EXECUTED DRAWINGS.
- S10. ADEQUATE PROVISION TO BE MADE FOR SCOURING AND SEDIMENTATION TO ALL DRAINAGE WORKS IN ACCORDANCE WITH COUNCIL'S REQUIREMENTS.
- S11. DRAINAGE LINES MUST BE INSTALLED AFTER SEWERAGE LINES HAVE BEEN INSTALLED WHERE SEWER IS PROPOSED ADJACENT TO DRAINAGE.

SERVICES ARE SHOWN INDICATIVE ONLY. CONTRACTOR TO CONFIRM ALL SERVICE LOCATIONS PRIOR TO CONSTRUCTION

DIMENSIONS SHALL NOT BE OBTAINED BY SCALING FROM THIS SET OF PLANS

No.	DESCRIPTION	DRN	APP	DATE
C	REVISED ARCHITECTURAL PLANS	A.C.	A.C.	06.07.18
D	DRAFT DA ISSUE	A.C.	A.C.	16.12.18
E	REVISED TO SUIT CONSULTANTS COMMENTS	A.C.	A.C.	19.12.18
F	REVISED TO PLANNERS COMMENTS	A.C.	A.C.	25.01.19
G	REVISED TO COUNCIL COMMENTS	M.D.	a.c	16.08.19



SURFACE ELEVATION DATA			
NUMBER	MINIMUM ELEVATION	MAXIMUM ELEVATION	COLOR
1	-3.00	-2.00	Red
2	-2.00	-1.00	Brown
3	-1.00	0.00	Green
4	0.00	1.00	Darker Green
5	1.00	2.25	Dark Green

Site Volume Table: Unadjusted

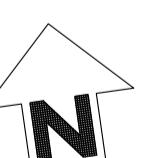
Site	Stratum	Surf1	Surf2	Cut	Fill	Net	Method
MILPERRA VILLAGE	Datum Surface	SURVEY - Top soil	11.589	8.356	3,232 (CUT)	TIN	

EARTHWORK VOLUME NOTES:

1. NO EXCAVATION FOR SERVICES HAS BEEN TAKEN INTO ACCOUNT.
2. 300mm DEEP FOOTINGS FOR BUILDINGS ON GROUND.
3. 300mm DEEP EXCAVATION FOR LANDSCAPING AND FOOTPATHS.
4. 340mm PAVEMENT THICKNESS.
5. 150mm THICK OF TOPSOIL.

NOTES:

1. IF ANY OF THE ABOVE ASSUMPTIONS CHANGE DURING CONSTRUCTION THE VOLUMES WILL CHANGE.
2. THE CURRENT GRADING IS BASED ON A FLOOD MODEL WHICH DOES NOT COMPLY WITH COUNCIL STANDARDS; THEREFORE, THE VOLUMES ARE SUBJECT TO CHANGE.



BULK EARTHWORKS PLAN

SCALE 1:400

0 10 15 20 25 Meters

SCALE : - 1 : 400 @ A1
- 1 : 800 @ A3

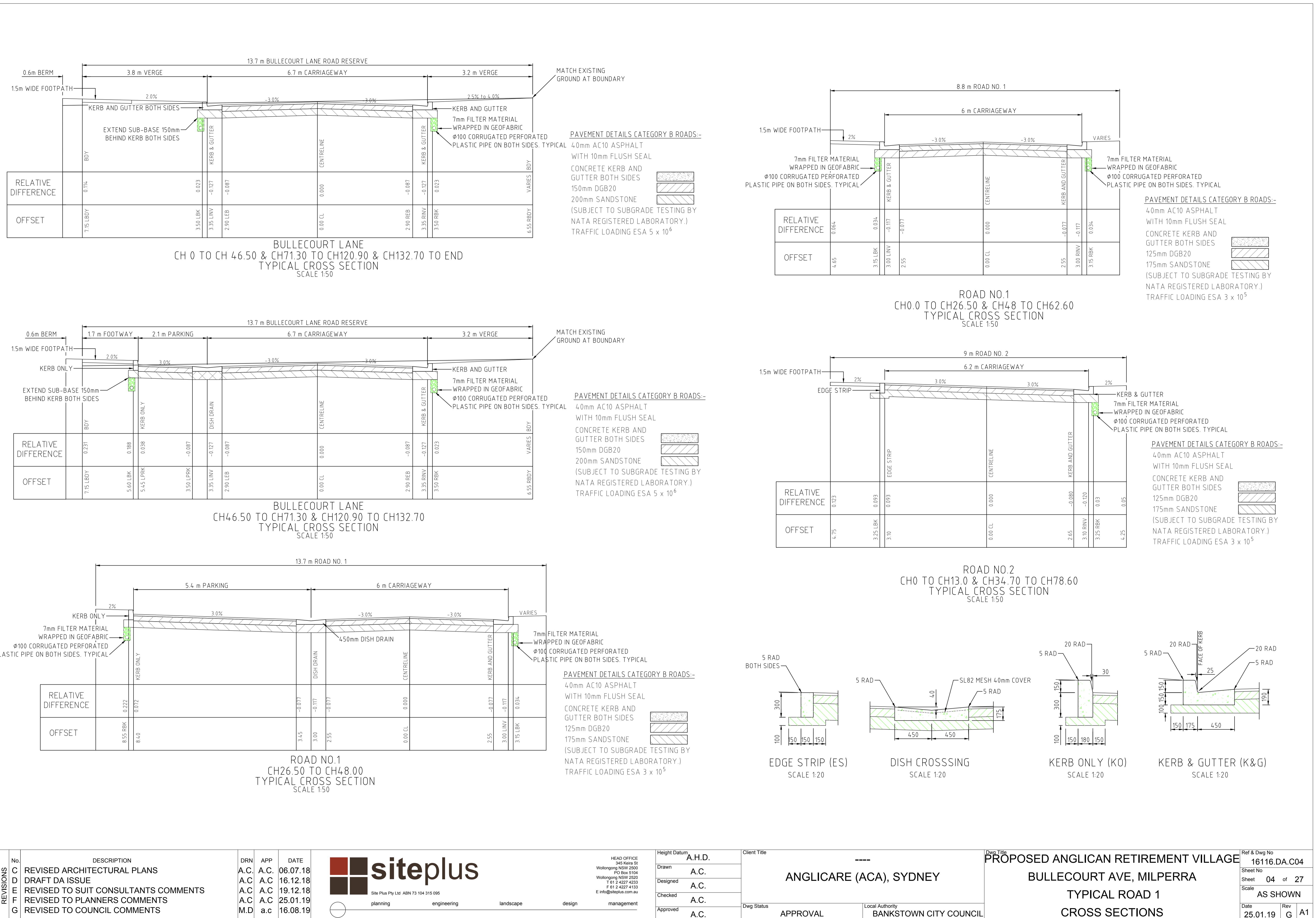
PROPOSED ANGLICAN RETIREMENT VILLAGE		Ref & Dwg No
		16116.DA.C03
Sheet No		03 of 27
Scale		1:500 @ A1
Date	25.01.19	Rev G A1
Height Datum	A.H.D.	Client Title
Drawn	A.C.	---
Designed	A.C.	---
Checked	A.C.	---
Approved	A.C.	Dwg Status APPROVAL Local Authority BANKSTOWN CITY COUNCIL

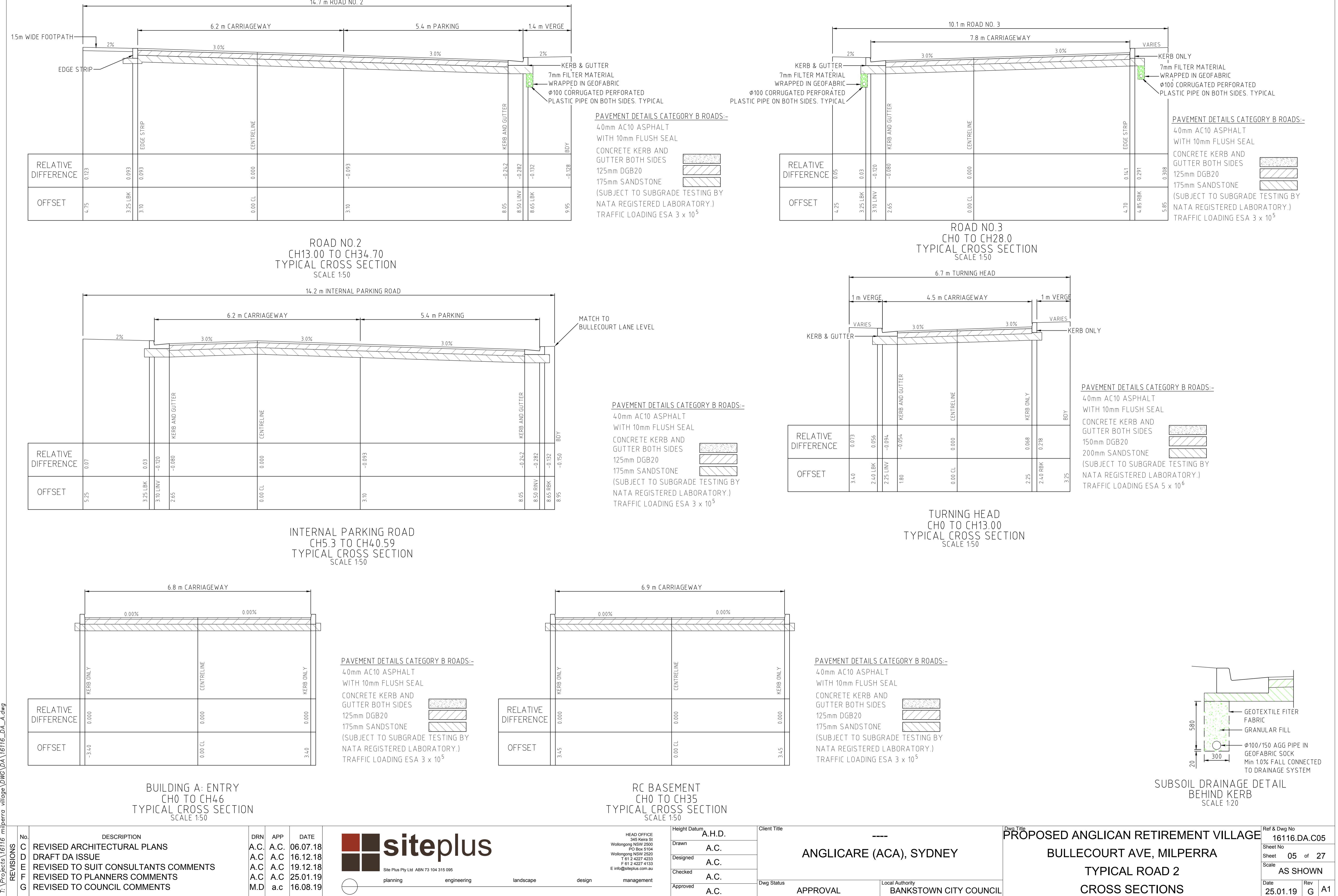
No. DESCRIPTION
C REVISED ARCHITECTURAL PLANS
D DRAFT DA ISSUE
E REVISED TO SUIT CONSULTANTS COMMENTS
F REVISED TO PLANNERS COMMENTS
G REVISED TO COUNCIL COMMENTS

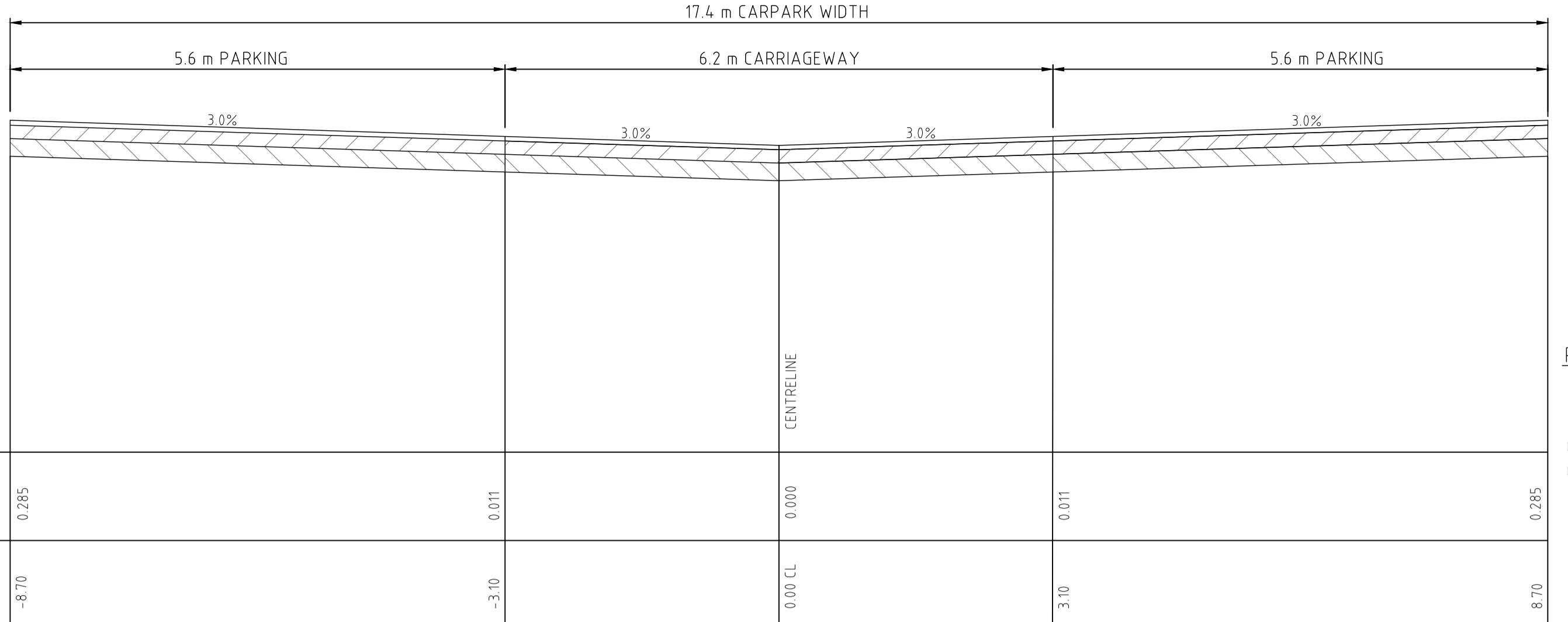
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Site Plus Pty Ltd ABN 73 104 315 095

planning engineering landscape design management

ANGLICARE (ACA), SYDNEY







RELATIVE DIFFERENCE

OFFSET

0.285

-3.10

0.011

BUILDING D CARPARK
CH0 TO CH35
TYPICAL CROSS SECTION
SCALE 1:50

PAVEMENT DETAILS CATEGORY B ROADS:-

7mm FILTER MATERIAL WRAPPED IN GEOFABRIC
 $\phi 100$ CORRUGATED PERFORATED PLASTIC PIPE ON BOTH SIDES. TYPICAL

RELATIVE DIFFERENCE

OFFSET

0.034

-0.117

0.090

0.240

0.261

PAVEMENT DETAILS CATEGORY B ROADS:-

- 40mm AC10 ASPHALT WITH 10mm FLUSH SEAL
- CONCRETE KERB AND GUTTER BOTH SIDES
- 125mm DGB20
- 175mm SANDSTONE
- (SUBJECT TO SUBGRADE TESTING BY NATA REGISTERED LABORATORY.)
- TRAFFIC LOADING ESA 3×10^5

BUILDING D ENTRY ROAD
CH0 TO CH79.4
TYPICAL CROSS SECTION
SCALE 1:50

RELATIVE DIFFERENCE

OFFSET

-4.90

-2.90

0.040

CHANNEL
CH0.0 TO CH129.0
TYPICAL CROSS SECTION
SCALE 1:50

RELATIVE DIFFERENCE

OFFSET

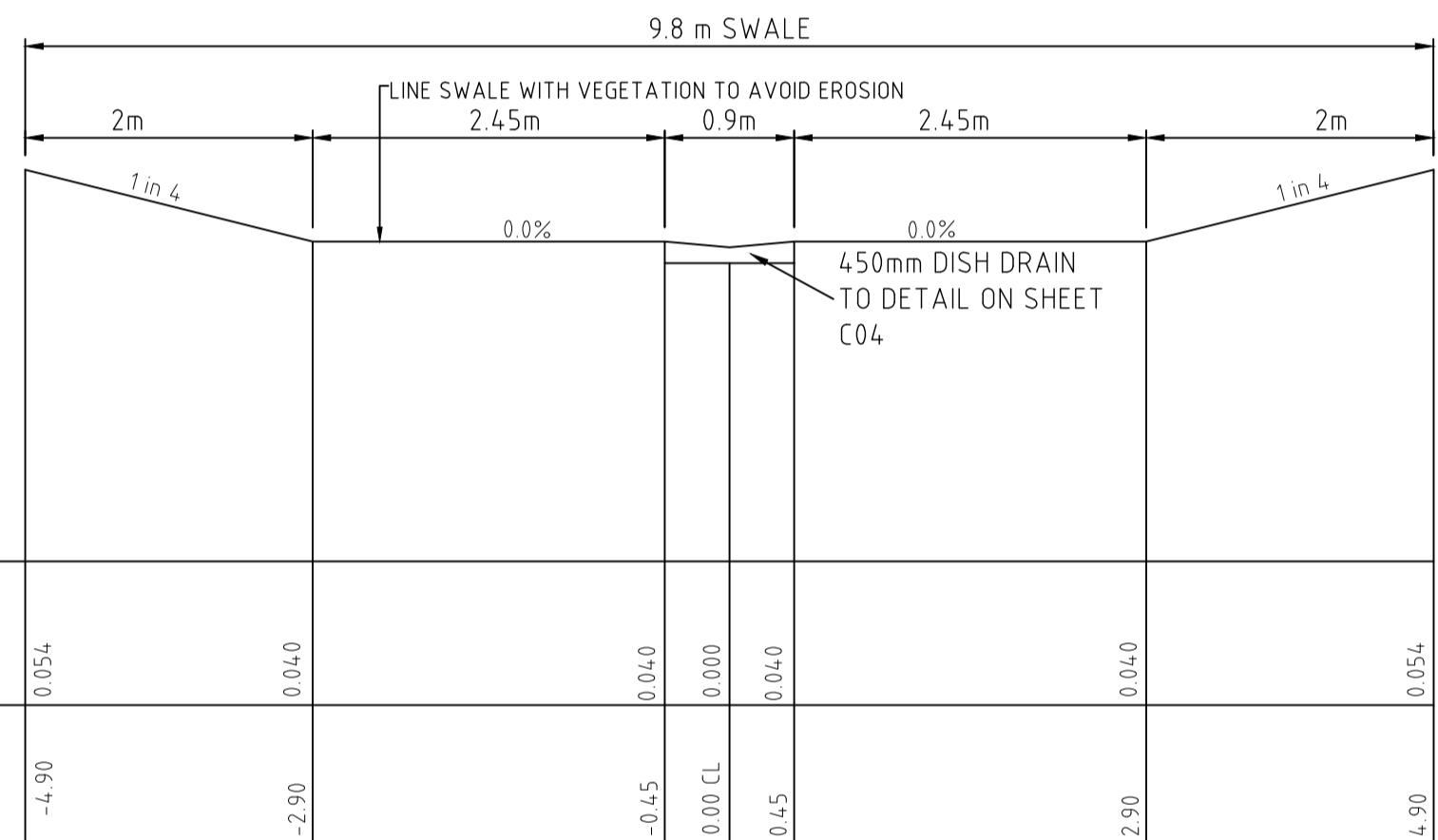
-4.90

-2.90

0.040

SWALE NO.2
CH0.0 TO CH53.0
TYPICAL CROSS SECTION
SCALE 1:50

REFER TO FLOOD STUDY AND STORMWATER MANAGEMENT PLAN
BY SITEPLUS FOR RESULTS OF THE FLOODWATER WITHIN SWALES
AND CHANNEL



RELATIVE DIFFERENCE

OFFSET

3.17

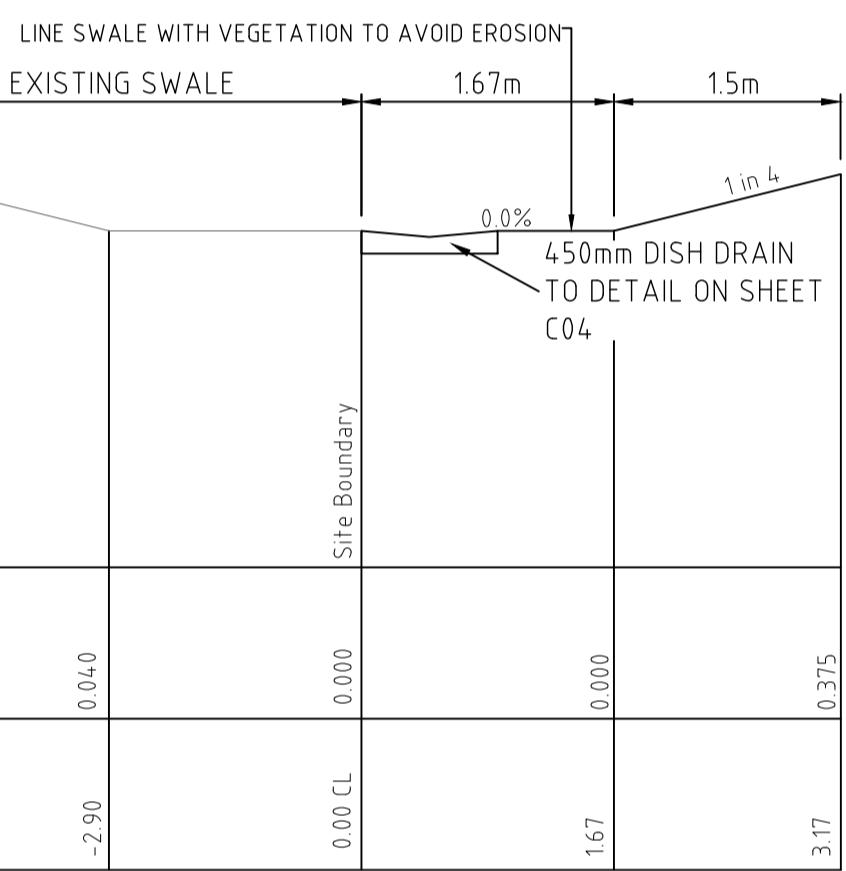
-2.90

0.040

0.000

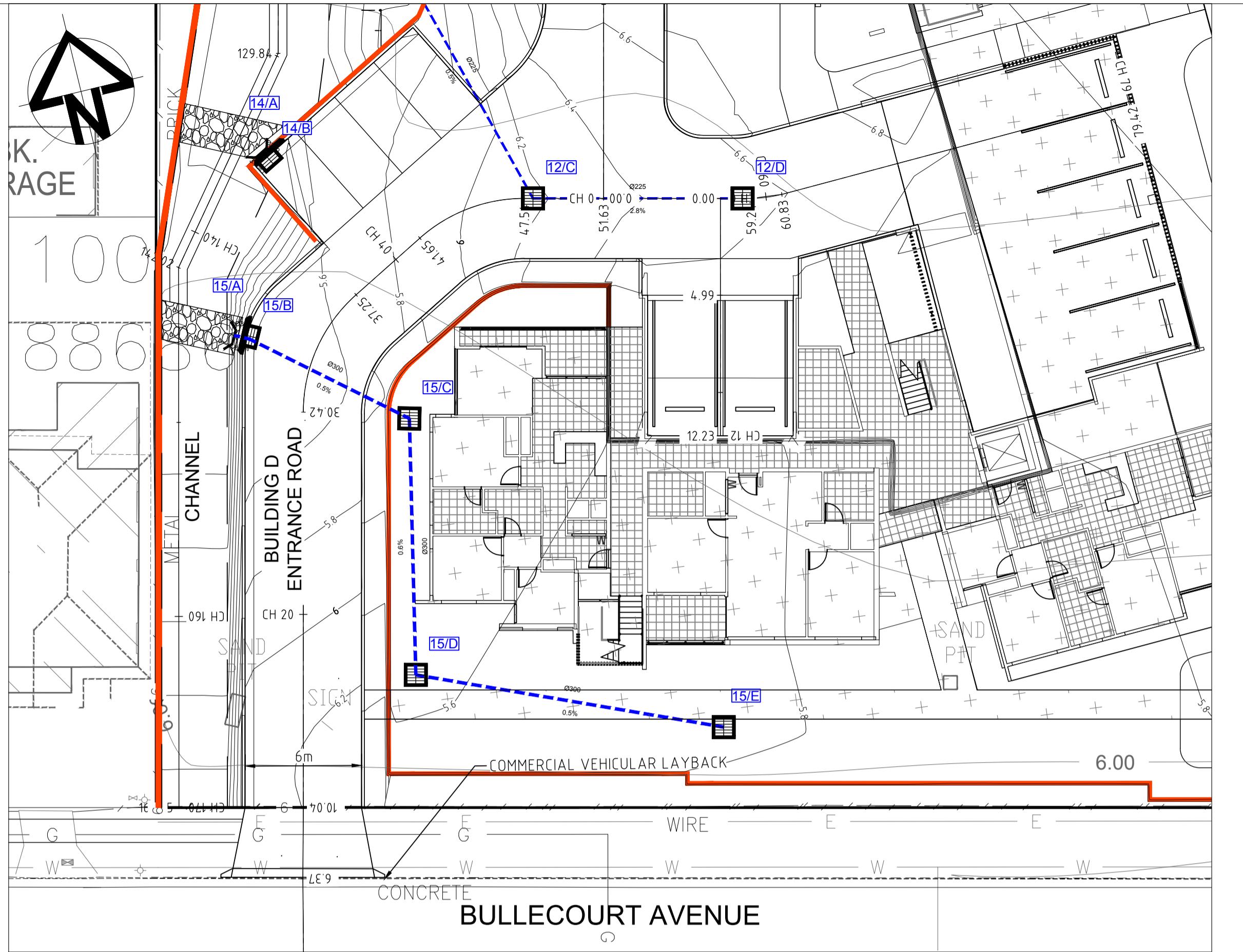
1.67

3.17



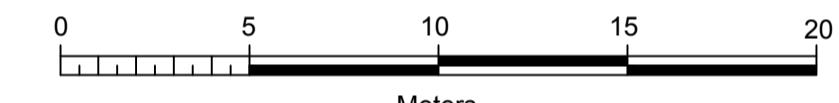
NORTHERN BDY CHANNEL
CH0.0 TO CH137.50
TYPICAL CROSS SECTION
SCALE 1:50

No.	DESCRIPTION	DRN	APP	DATE
C	REVISED ARCHITECTURAL PLANS	A.C.	A.C.	06.07.18
D	DRAFT DA ISSUE	A.C.	A.C.	16.12.18
F	REVISED TO SUIT CONSULTANTS COMMENTS	A.C.	A.C.	19.12.18
M	REVISED TO PLANNERS COMMENTS	A.C.	A.C.	25.01.19
G	REVISED TO COUNCIL COMMENTS	M.D.	a.c	16.08.19

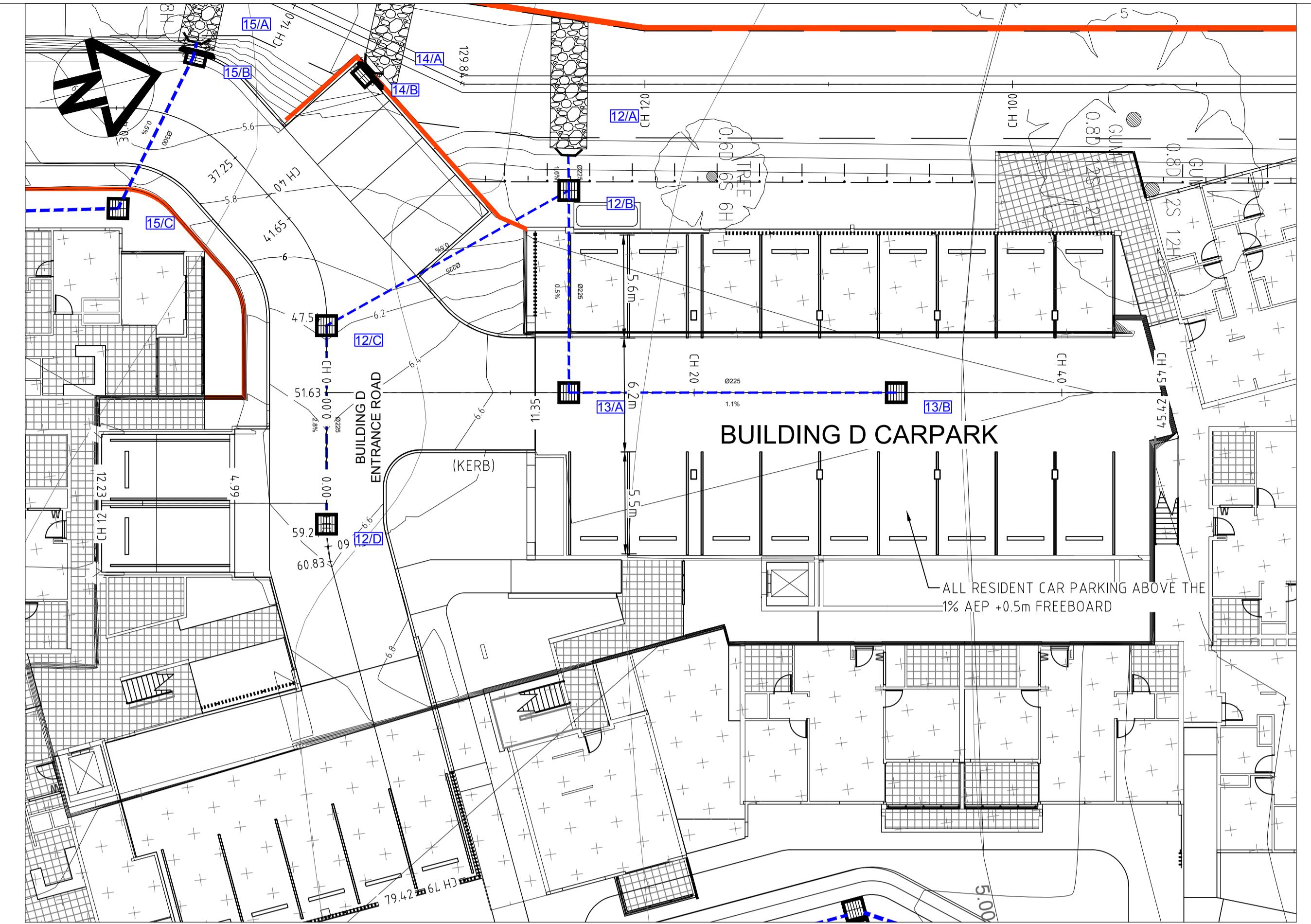
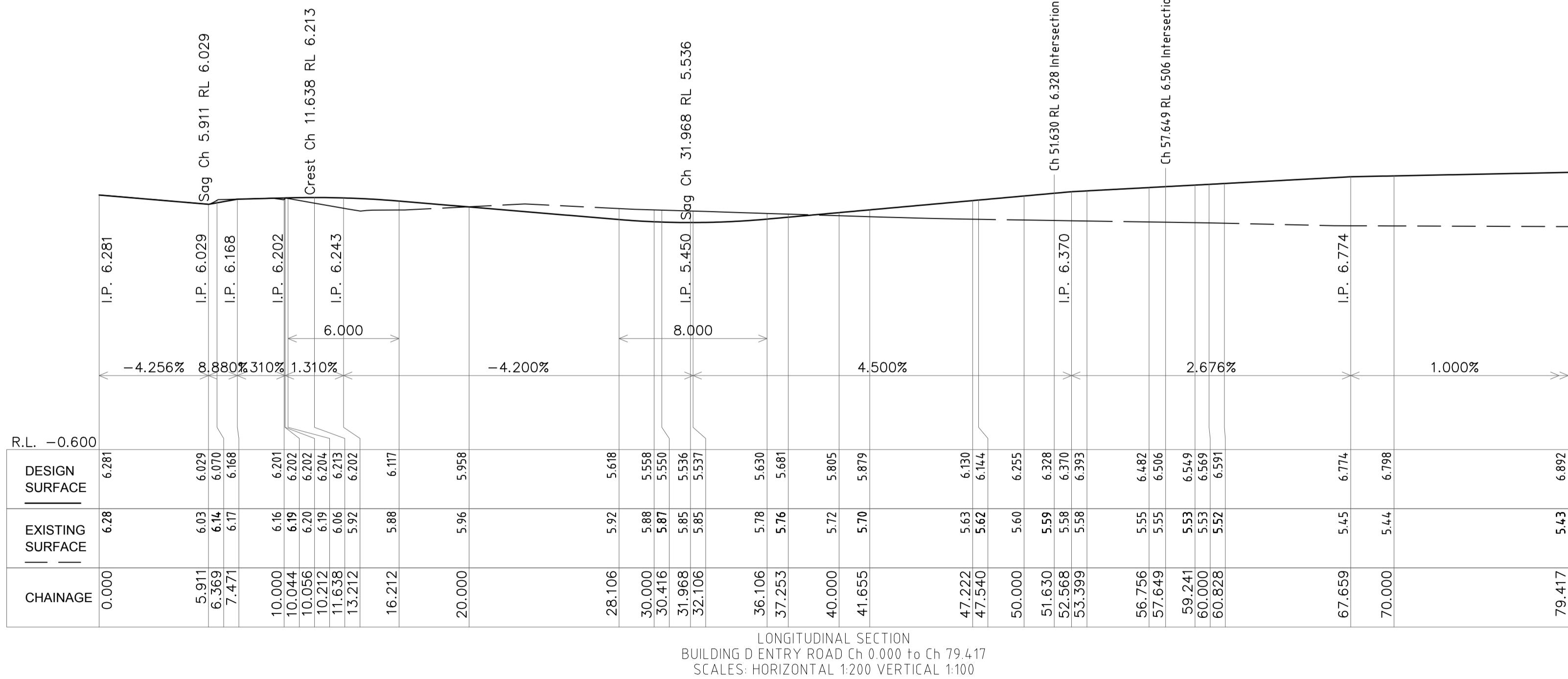


BUILDING D ENTRANCE ROAD
CH0 TO CH79.00

SCALE 1:200



SCALE : - 1 : 200 @ A1
- 1 : 400 @ A3

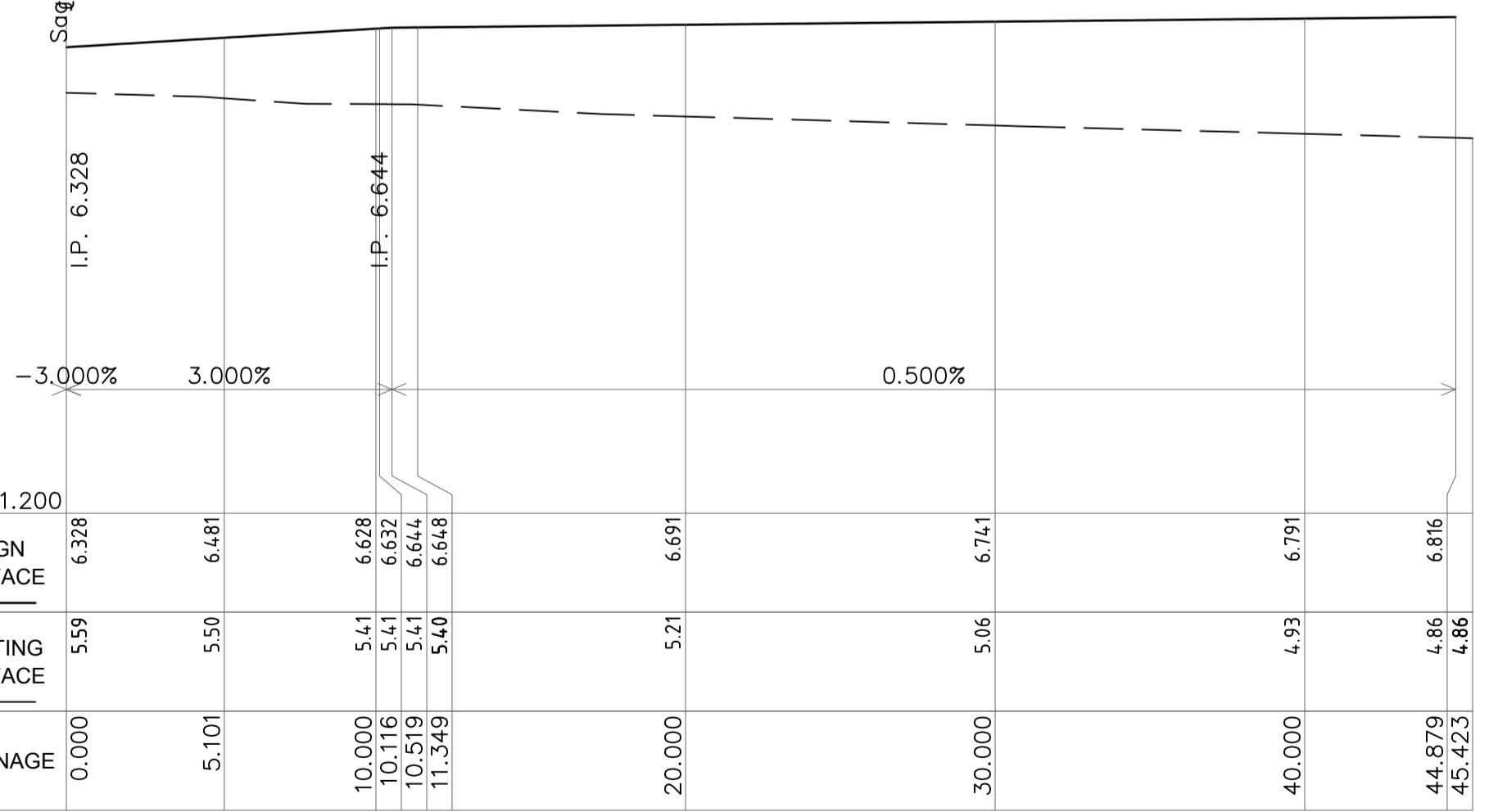


BUILDING D CARPARK
CH0 TO CH45.42

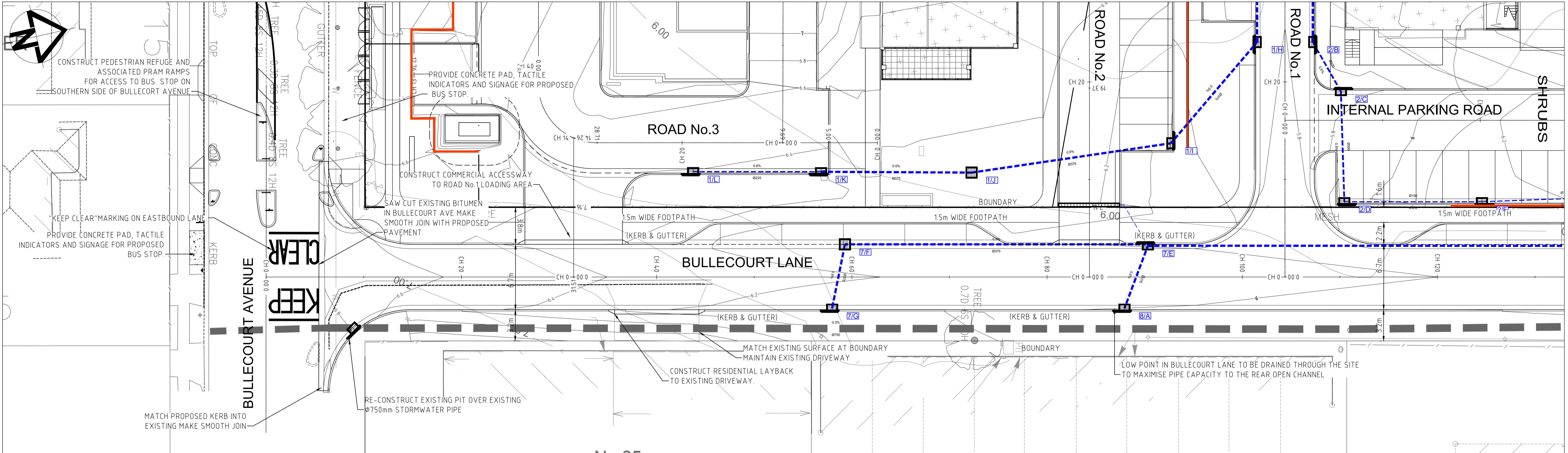
SCALE 1:200



SCALE : - 1 : 200 @ A1
- 1 : 400 @ A3



G	T	M	D	C	Z	DESCRIPTION
						REVISED ARCHITECTURAL PLANS
						DRAFT DA ISSUE
						REVISED TO SUIT CONSULTANTS COMMENTS
						REVISED TO PLANNERS COMMENTS
						REVISED TO COUNCIL COMMENTS

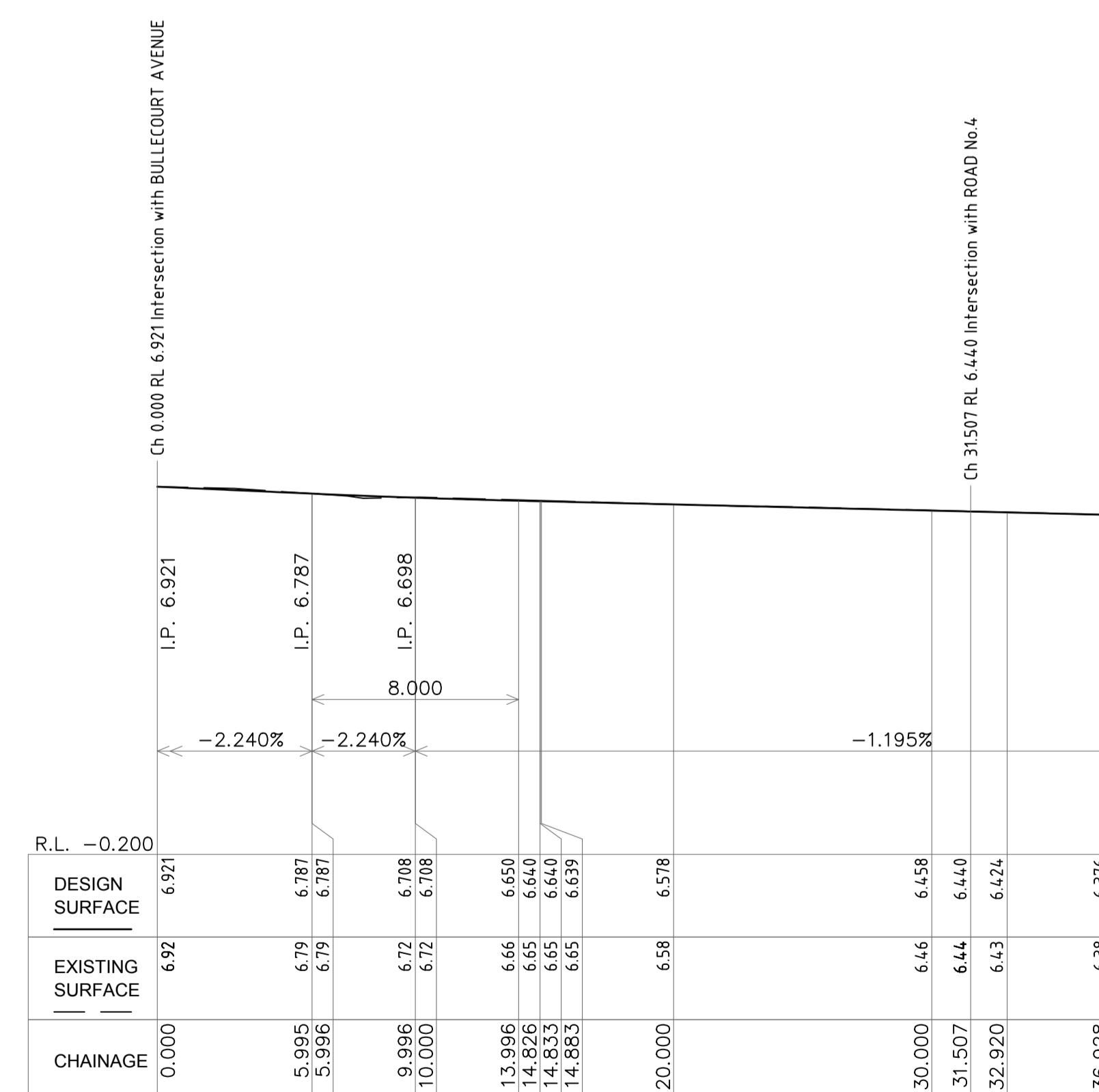


BULLECOURT LANE CH0 TO CH100

SCALE 1:200



Meters



LONGITUDINAL SECTION
BULLECOURT LANE Ch 0.000 to Ch 100.000
SCALES: HORIZONTAL 1:200 VERTICAL 1:100

REVISIONS	No.	DESCRIPTION
	C	REVISED ARCHITECTURAL PLANS
	D	DRAFT DA ISSUE
	E	REVISED TO SUIT CONSULTANTS COMMENTS
	F	REVISED TO PLANNERS COMMENTS
	G	REVISED TO COUNCIL COMMENTS



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Height Datum
Drawn
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Approved

A.H.D.
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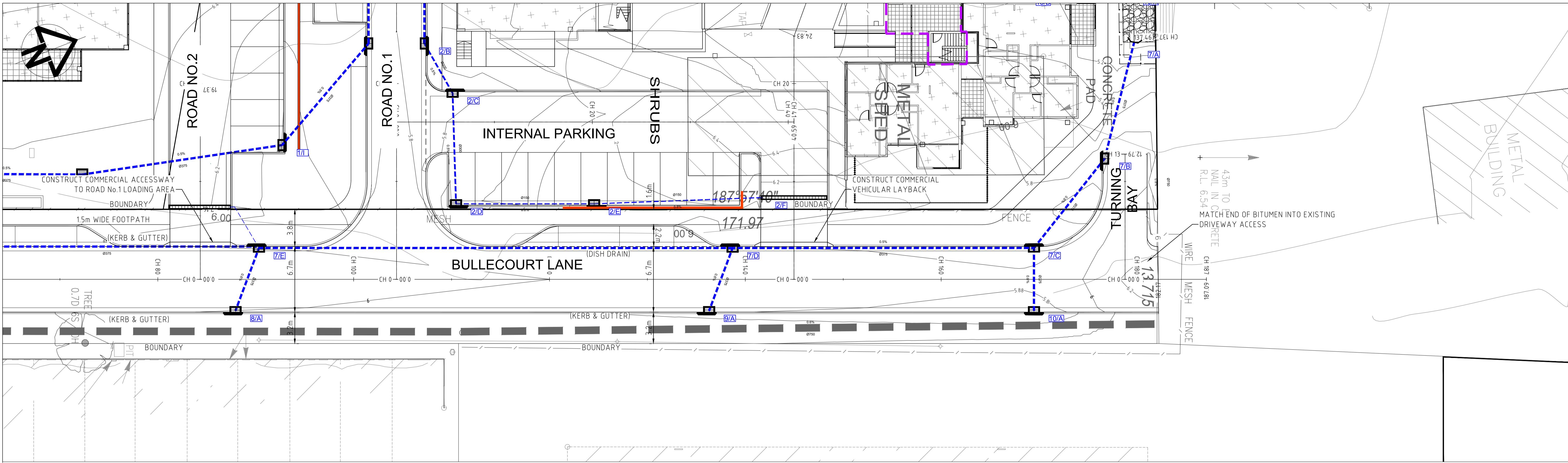
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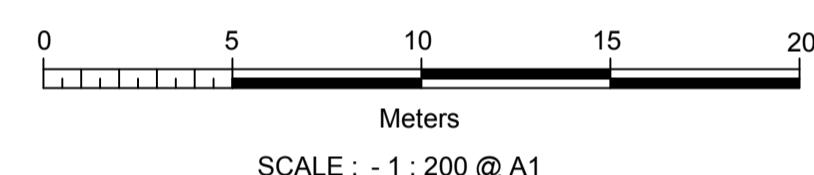
ANGLICARE (ACA), SYDNEY

Dwg Title
PROPOSED ANGLICAN RETIREMENT VILLAGE
BULLECOURT AVE, MILPERRA
BULLECOURT LANE CH0 TO CH100
LONGSECTION

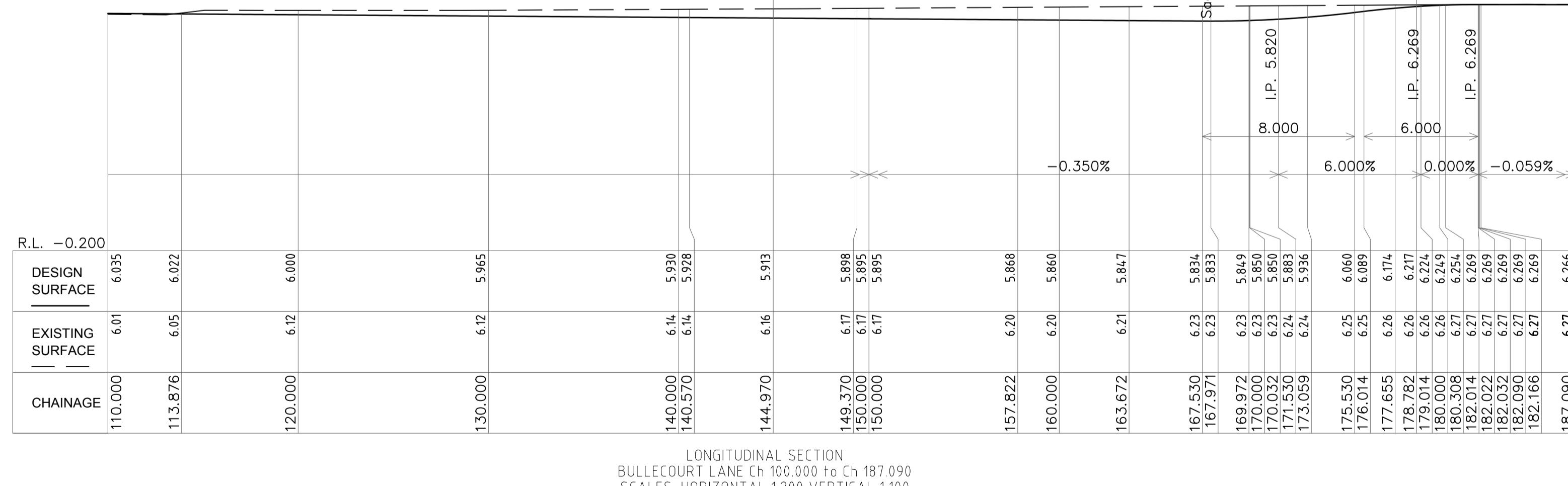


BULLECOURT LANE
CH100 TO CH187.09

SCALE 1:200



with BASEMENT ENTRY



REVISIONS	No.	DESCRIPTION
	C	REVISED ARCHITECTURAL PLANS
	D	DRAFT DA ISSUE
	E	REVISED TO SUIT CONSULTANTS COMMENTS
	F	REVISED TO PLANNERS COMMENTS
	G	REVISED TO COUNCIL COMMENTS



SCALES: HORIZONTAL 1:200 VERTICAL 1:100

DOUGLASCOURT LANE CH 100.000 TO CH 107.000
SCALES: HORIZONTAL 1:200 VERTICAL 1:100

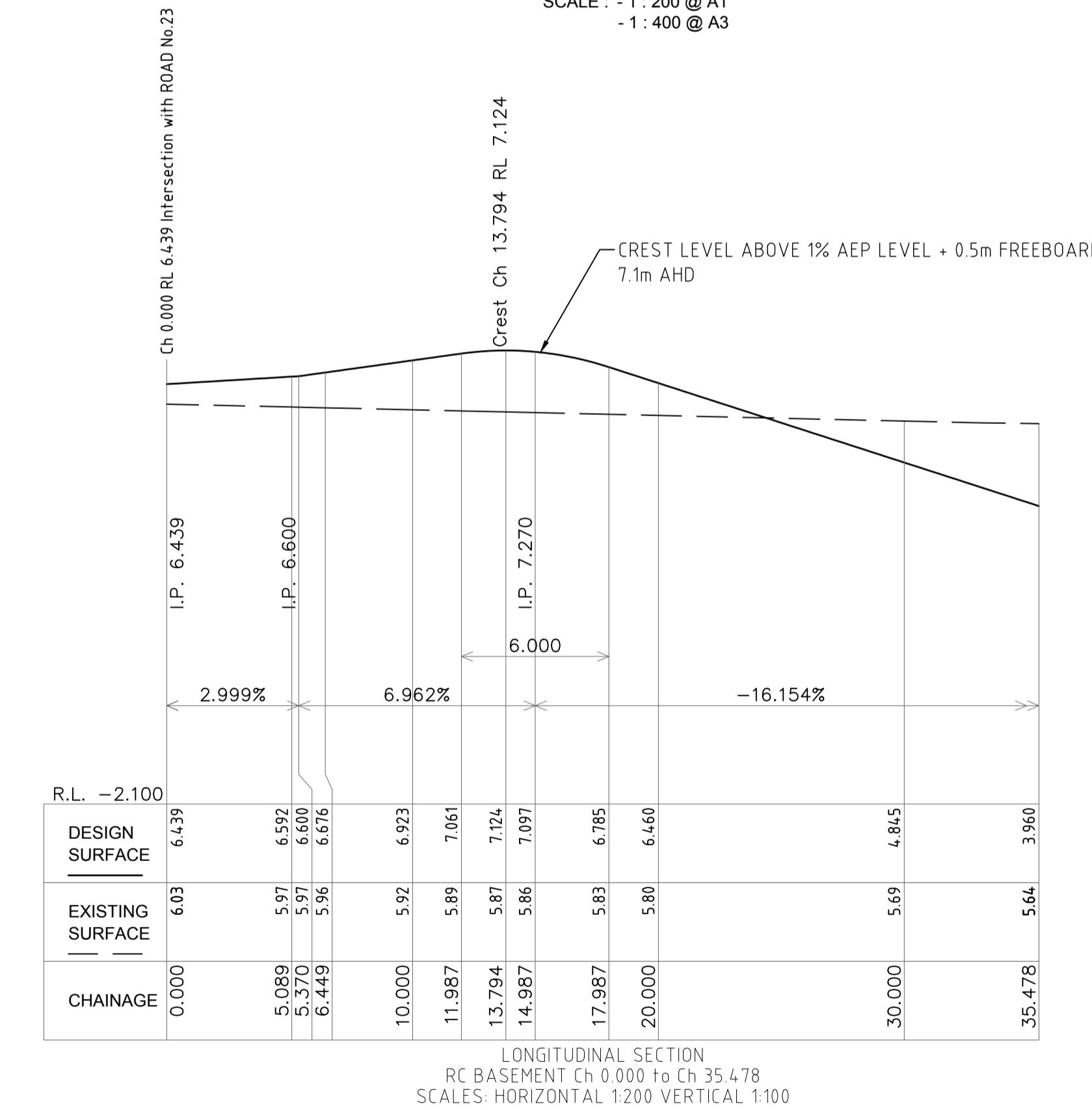
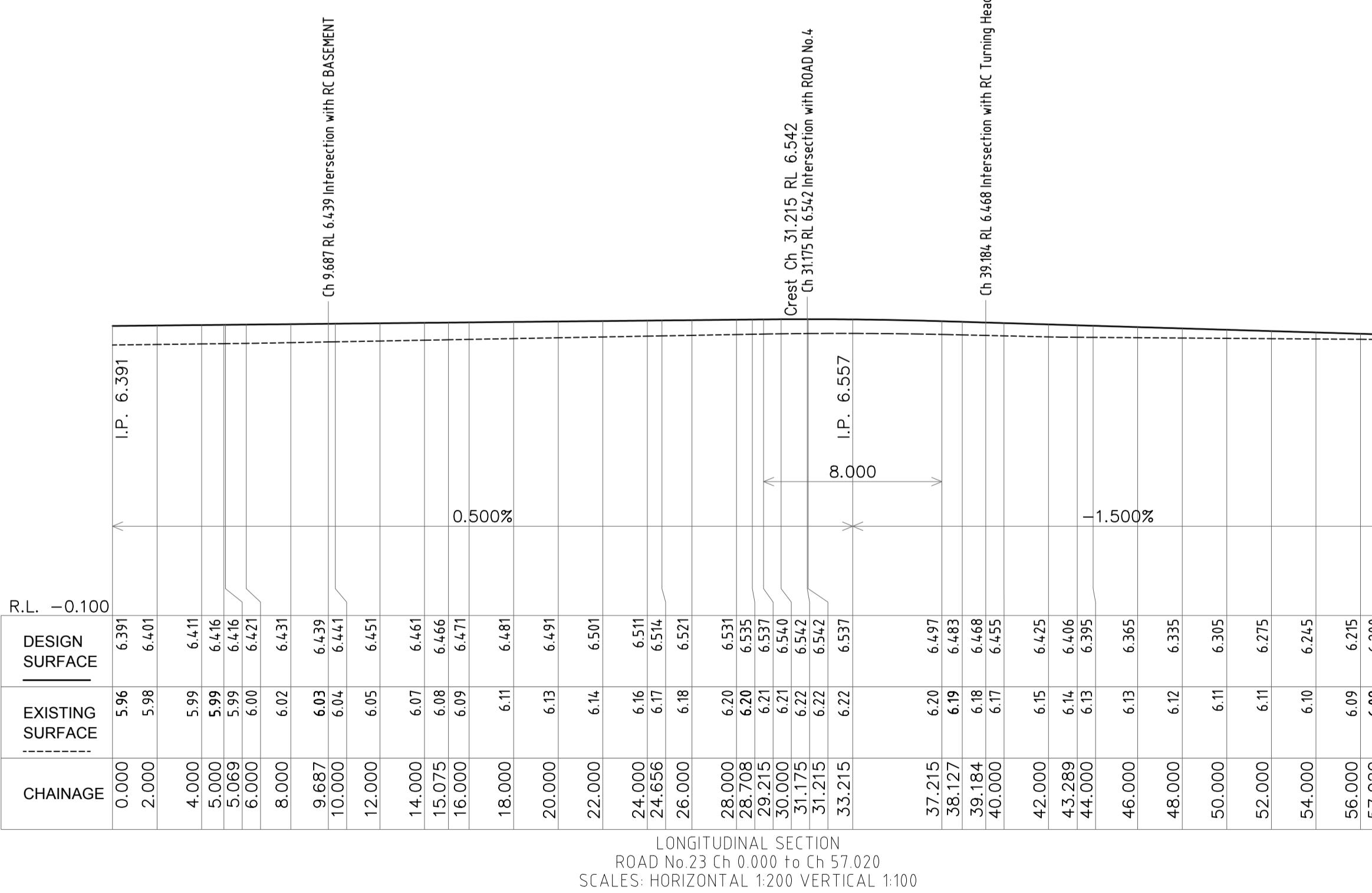
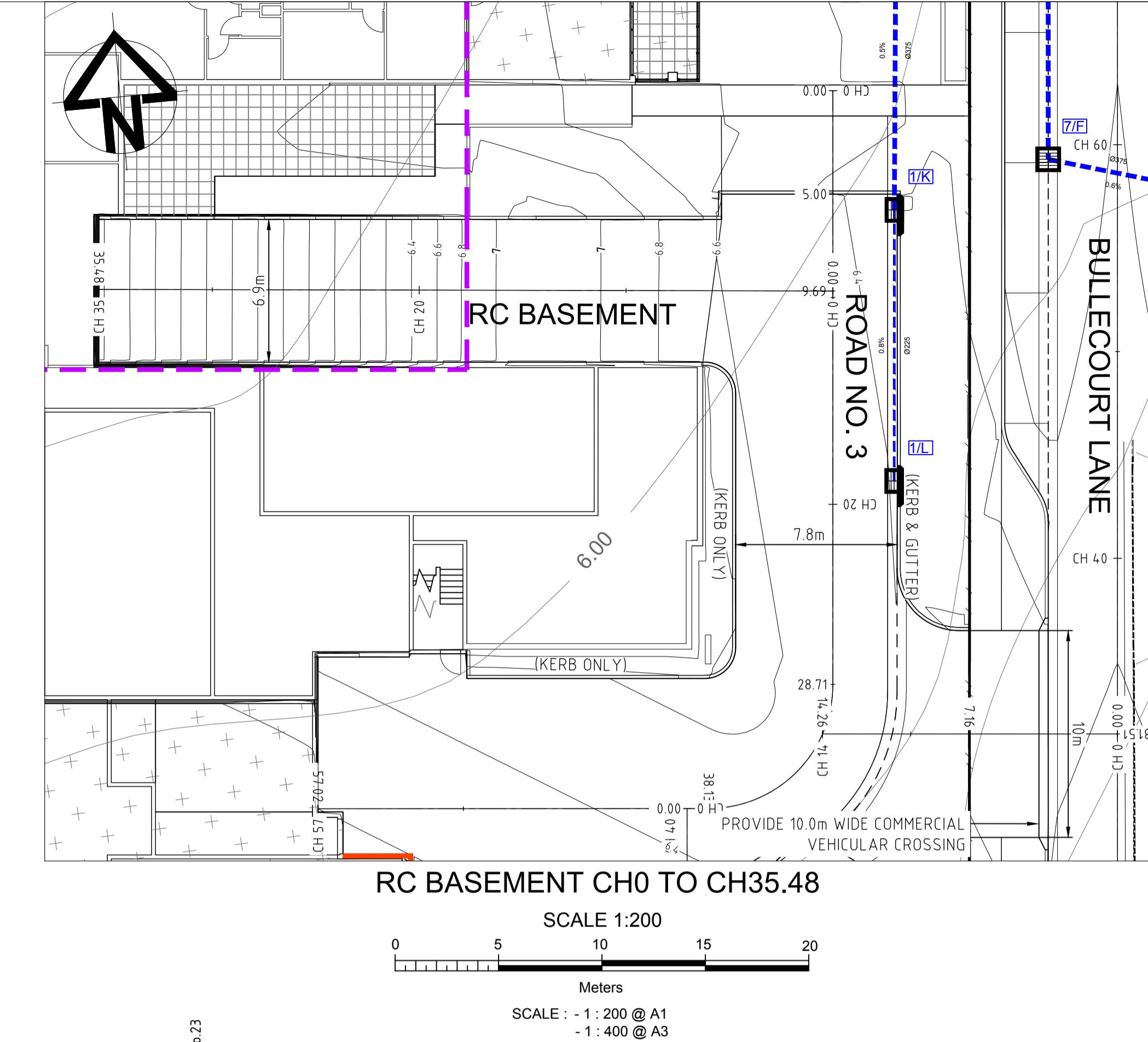
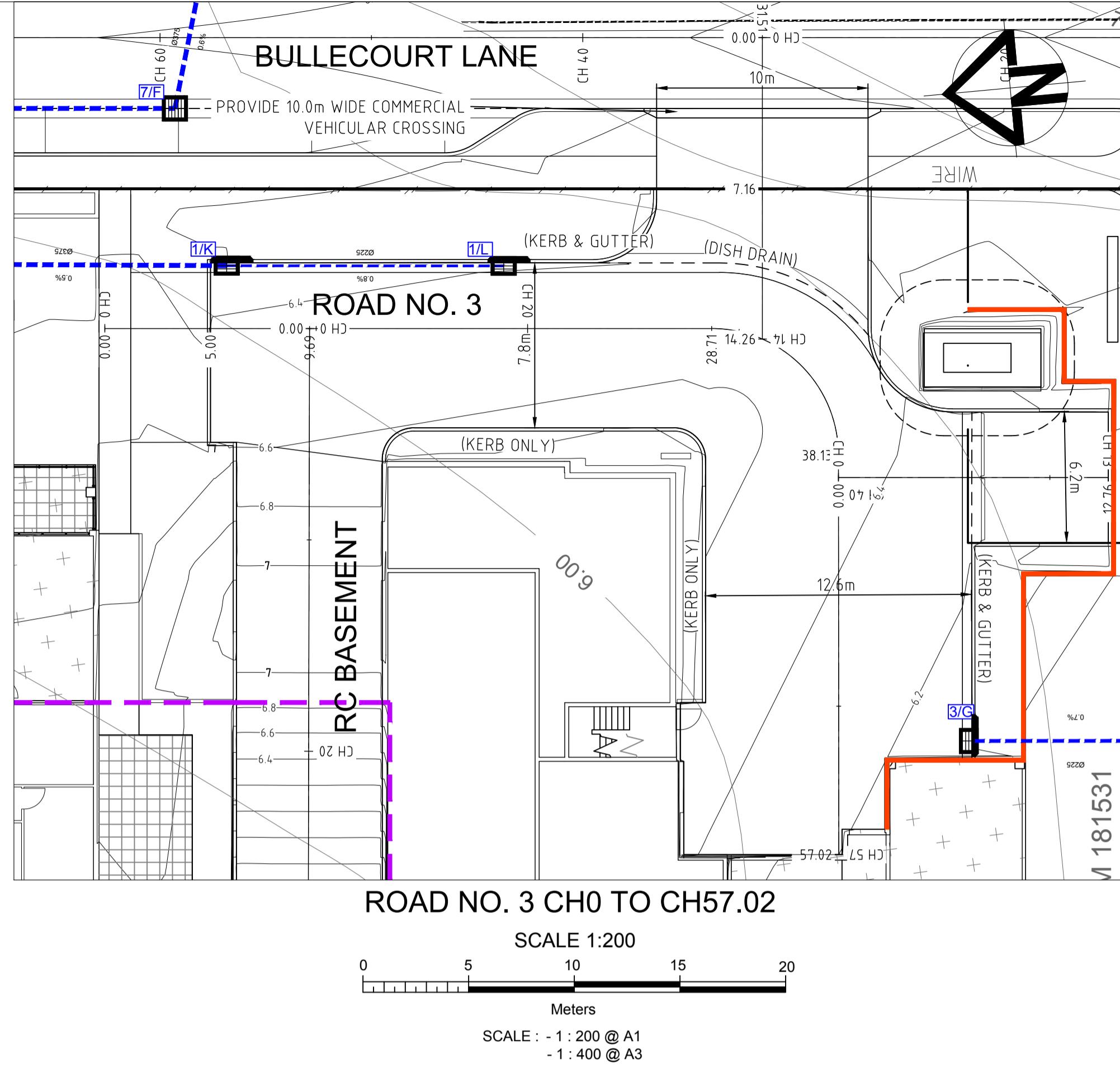
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Drawn A.C.		
Designed A.C.		
Checked A.C.		
Approved A.C.	Dwg Status APPROVAL	Local Authority BANKSTOWN CITY COUNCIL

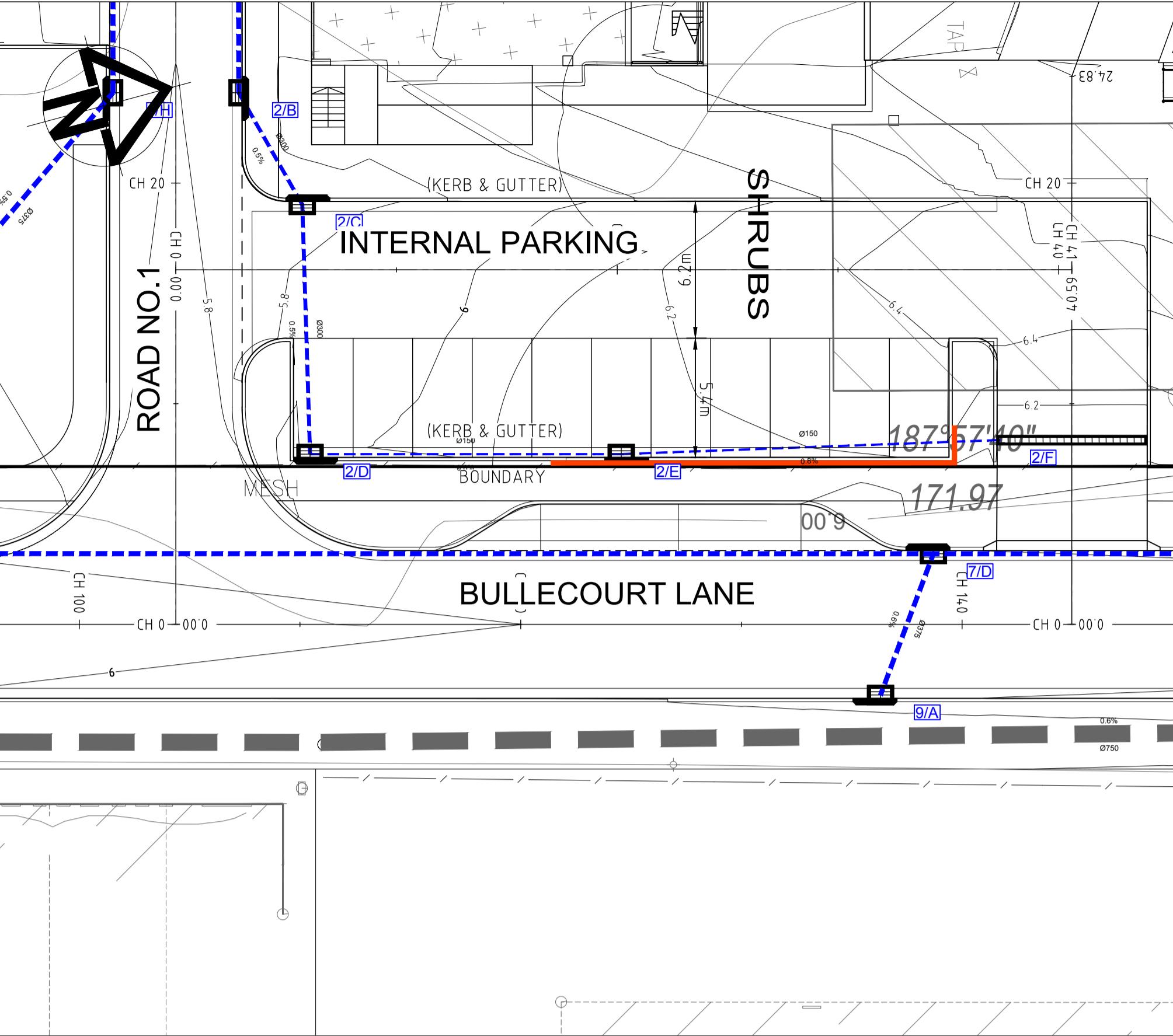
ANGLICARE (ACA), SYDNEY

Dwg Title
PROPOSED ANGLICAN RETIREMENT VILLAGE
BULLECOURT AVE, MILPERRA
BULLECOURT LANE CH100 TO CH187
LONGSECTION

Ref & Dwg No	16116.DA.C09		
Sheet No	09	of	27
Scale	1:200 @ A1		
Date	Rev	A1	
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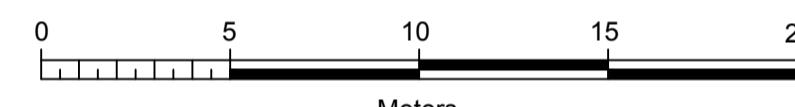
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DRAFT DA ISSUE
REVISED TO SUIT CONSULTANTS COMMENTS
REVISED TO PLANNERS COMMENTS
REVISED TO COUNCIL COMMENTS



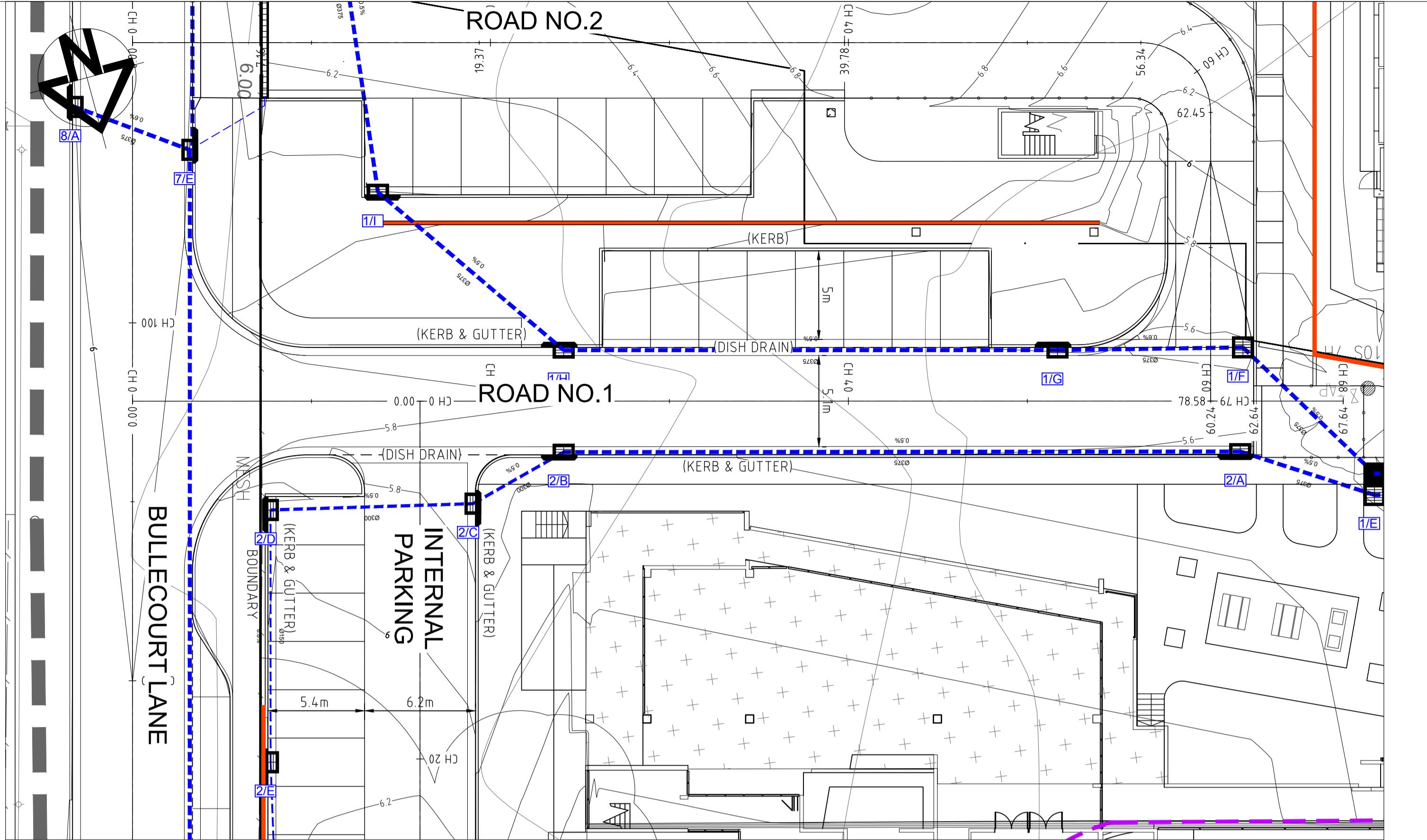
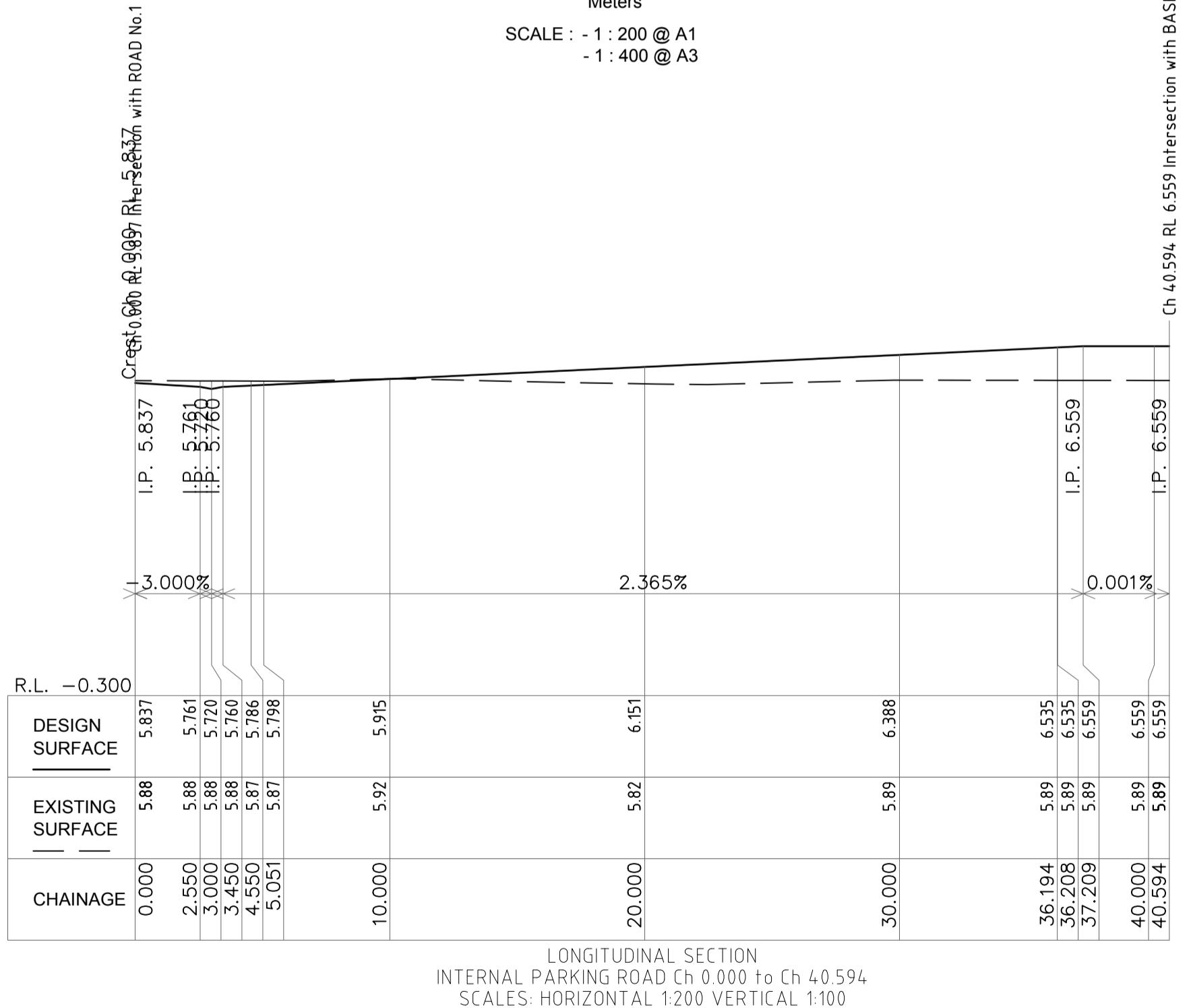


INTERNAL PARKING CH0 TO CH40.59

SCALE 1:200

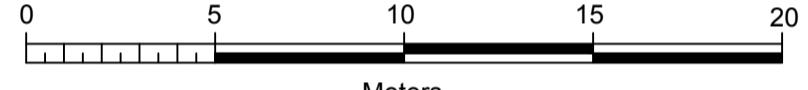


SCALE : - 1 : 200 @ A1
- 1 : 400 @ A3

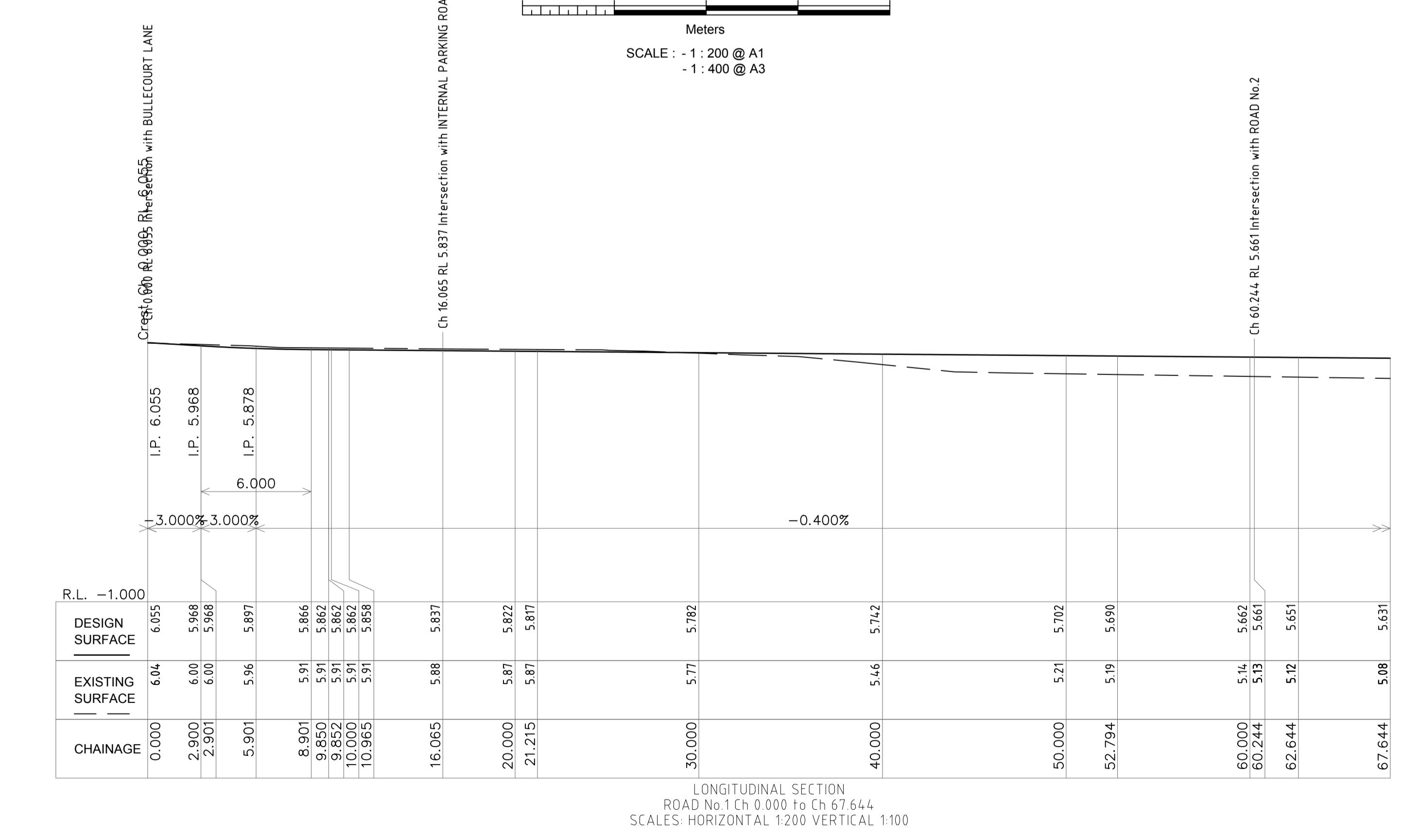


ROAD NO.1 CH0 TO CH67.64

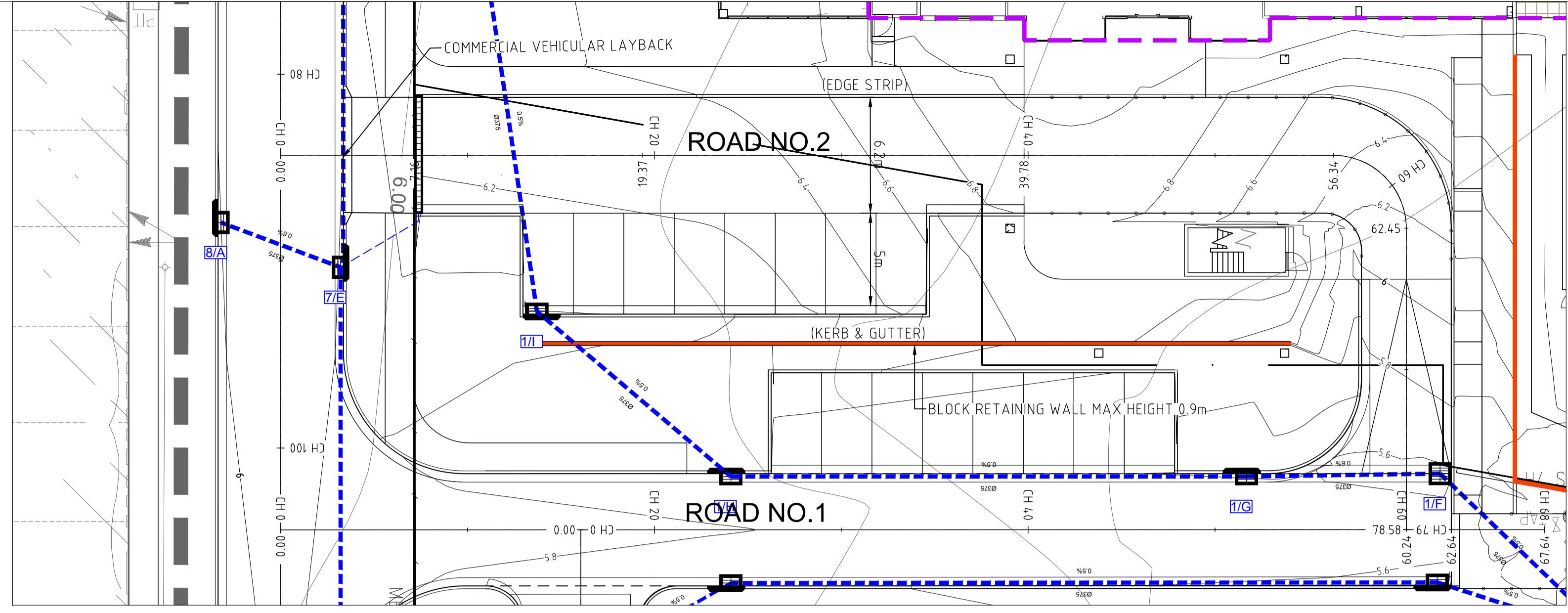
SCALE 1:200



SCALE : - 1 : 200 @ A1
- 1 : 400 @ A3



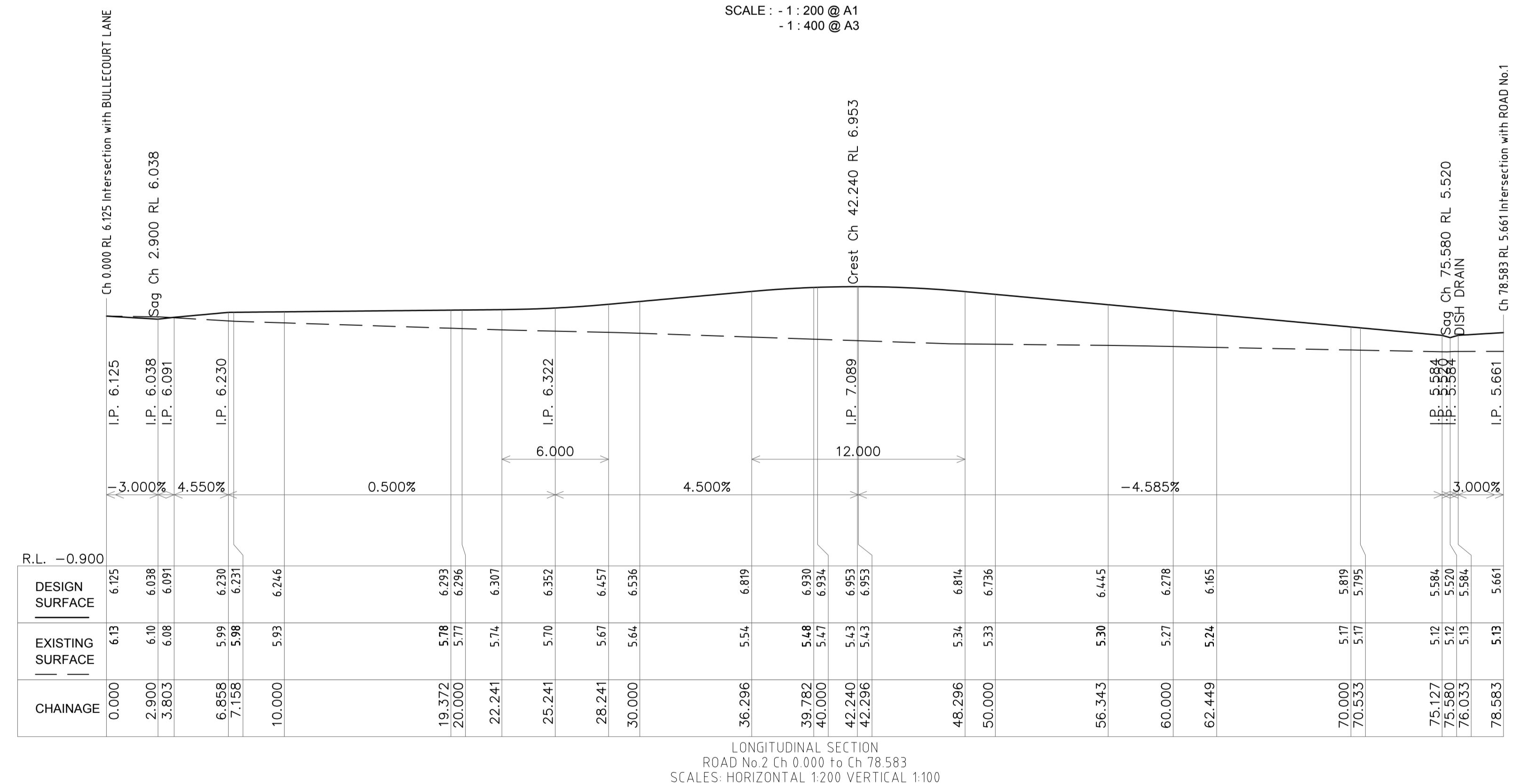
No.	C.Z.	DESCRIPTION
G	T	REVISED ARCHITECTURAL PLANS
T	M	DRAFT DA ISSUE
C	S	REVISED TO SUIT CONSULTANTS COMMENTS
A	C	REVISED TO PLANNERS COMMENTS
Z	O	REVISED TO COUNCIL COMMENTS



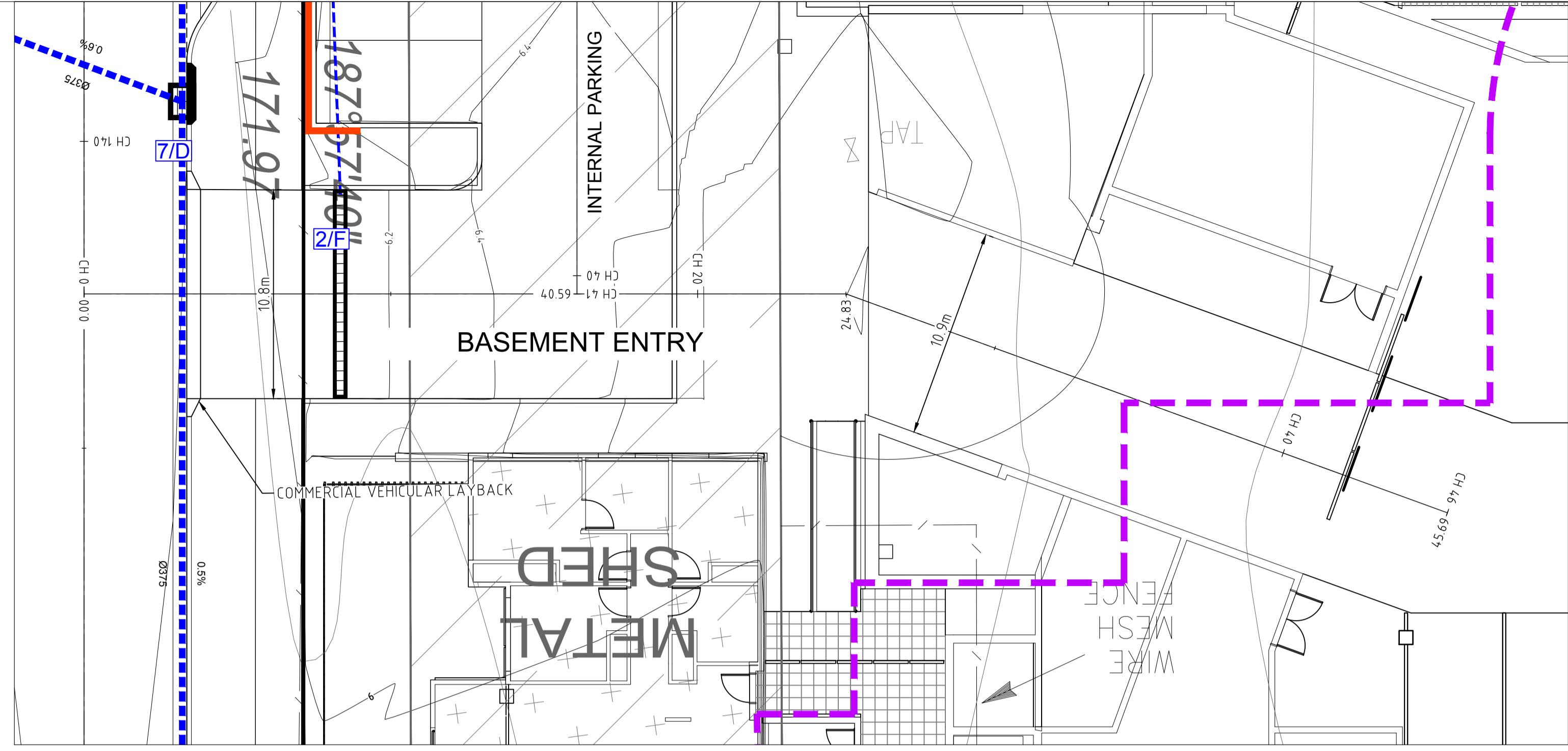
ROAD NO.2 CH0 TO CH78.583

SCALE 1:200
0 5 10 15 20
Meters

SCALE : - 1 : 200 @ A1
- 1 : 400 @ A3



DESCRIPTION		
REVISED ARCHITECTURAL PLANS	DRN	APP
DRAFT DA ISSUE	A.C.	A.C.
REVISED TO SUIT CONSULTANTS COMMENTS	A.C.	A.C.
REVISED TO PLANNERS COMMENTS	A.C.	A.C.
REVISED TO COUNCIL COMMENTS	M.D.	a.c.
DATE	06.07.18	16.12.18
	19.12.18	25.01.19
	25.01.19	16.08.19

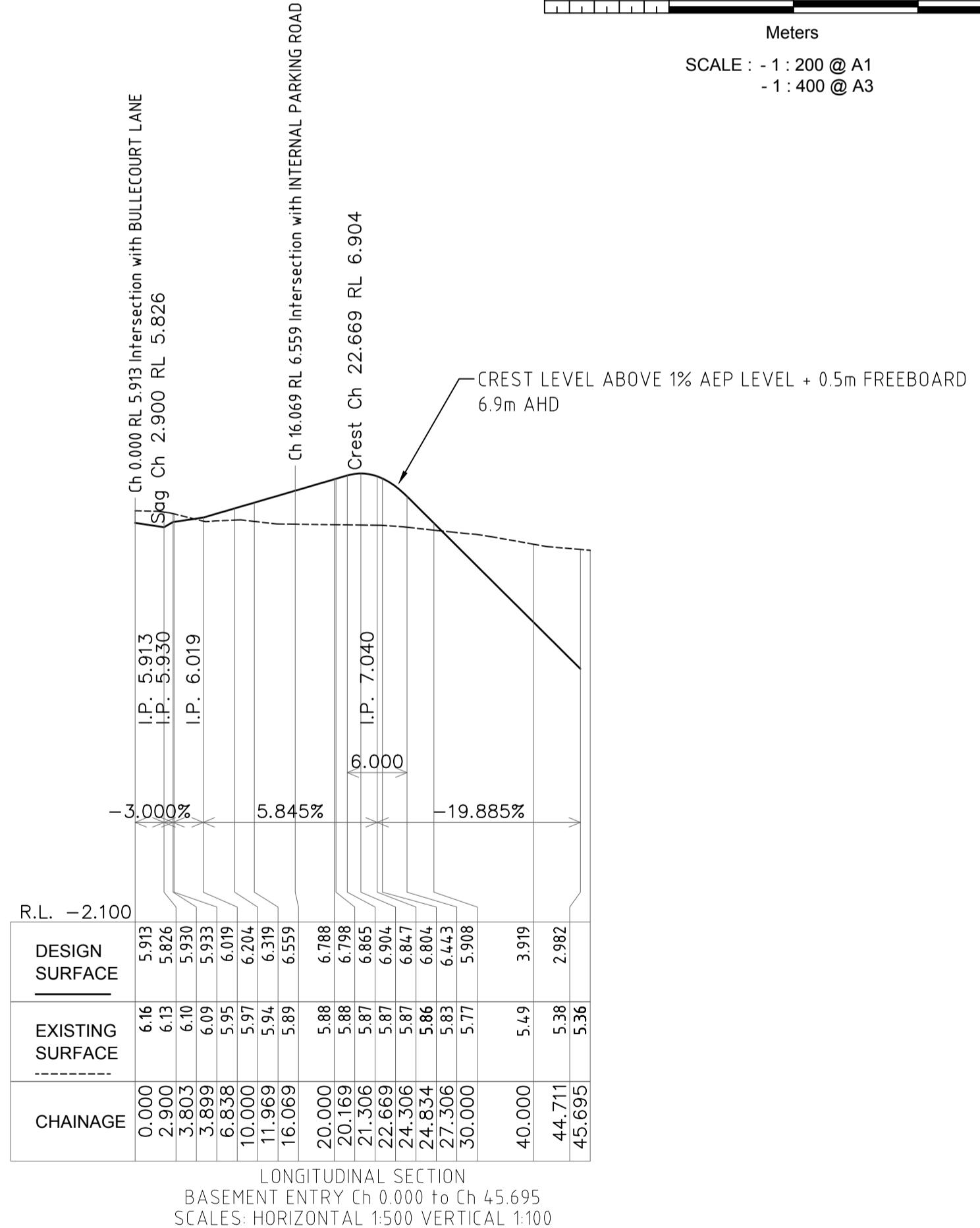


BASEMENT ENTRY CH0 TO CH46.000

SCALE 1:200

0 5 10 15 20
Meters

SCALE : - 1 : 200 @ A1
- 1 : 400 @ A3



N	Z.	DESCRIPTION
G	T	REVISED ARCHITECTURAL PLANS
M	D	DRAFT DA ISSUE
C		REVISED TO SUIT CONSULTANTS COMMENTS
		REVISED TO PLANNERS COMMENTS
		REVISED TO COUNCIL COMMENTS



PIT CATCHMENT PLAN

REVISIONS			DESCRIPTION
	C		REVISED ARCHITECTURAL PLANS
	D		DRAFT DA ISSUE
	E		REVISED TO SUIT CONSULTANTS COMMENTS
	F		REVISED TO PLANNERS COMMENTS
	G		REVISED TO COUNCIL COMMENTS



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E info@siteplus.com

Height Datum	A.H.D.	Client Title	----
Drawn	A.C.		
Designed	A.C.		
Checked	A.C.		
Approved	A.C.	Dwg Status	Local Authority
		APPROVAL	BANKSTOWN CITY COUNCIL

UPSTREAM PIT CATCHMENT PLAN

**PROPOSED ANGLICAN RETIREMENT VILLAGE
BULLECOURT AVE, MILPERRA**

PIT CATCHMENT PLAN

Ref & Dwg No			
16116.DA.C14			
Sheet No			
Sheet	14	of	27
Scale			
1:500 @ A1			
Date	Rev		
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Z		DESCRIPTION	
C		REVISED ARCHITECTURAL PLANS	
D		DRAFT DA ISSUE	
F		REVISED TO SUIT CONSULTANTS COMMENTS	
M		REVISED TO PLANNERS COMMENTS	
G		REVISED TO COUNCIL COMMENTS	

		HGL Report - Proposed											Return Period: 10yrs		Location: Milperra										
		Pipe Connecting Pits (Downstream Upstream)	Pipe ID	Pipe Class	Pipe Diameter	Pipe Length	Pipe Design Flow	Mannings n	Pipe Velocity	Pipe Part Velocity	Pipe Velocity Head	HGL at Downstream Pit	Pipe Friction Slope	HGL at Upstream Pit	Pit Loss Coefficient	Pipe Head Loss	Adopted Upstream Water Level	Pit Surcharge Level (Pit Inlet/Outlet level)	Downstream Pipe Invert	Upstream Pipe Invert	Pipe Slope	Pipe Design Flow	Pipe HGL Capacity	Pipe Manning Capacity	
					(mm)	(m)	(l/s)		(m/s)	(m/s)	(m)	(%)	(m)		(m)	Ku	(m)	(m)	(m)	(m)	(m)	(%)	(l/s)	(l/s)	
1/A 1/B	1/B1/A	Class 3 RRJ	375	1723	628.1	0.013	1.90	1.896	0.183	4.735	0.01	0.013	4.748	1.754	0.322	5.070	5.064	4.735	4.745	4.360	4.370	0.581	628.1	550.8	400.8
1/B 1/C	1/C1/B	Class 3 RRJ	375	5.894	347.9	0.013	1.58	1.575	0.127	5.064	0.01	0.031	5.095	1.033	0.131	5.226	5.309	4.775	4.805	4.400	4.430	0.509	347.9	342.9	250.2
1/C 1/D	1/D1/C	Class 3 RRJ	375	14.726	286.2	0.013	1.30	1.296	0.086	5.226	0.00	0.053	5.279	1.373	0.118	5.397	5.422	4.835	4.915	4.460	4.540	0.543	286.2	354.7	258.5
1/D 1/E	1/E1/D	Class 4 RRJ	375	23.369	222.2	0.013	1.28	1.284	0.052	5.397	0.00	0.052	5.449	1.461	0.075	5.524	5.595	4.945	5.065	4.570	4.690	0.514	222.2	344.5	251.3
1/E 1/F	1/F1/E	Class 4 RRJ	375	11.034	159.6	0.013	1.44	1.445	0.107	5.524	0.00	0.049	5.573	1.500	0.160	5.733	5.537	5.095	5.155	4.720	4.780	0.544	159.6	177.5	129.3
1/F 1/G	1/G1/F	Class 4 RRJ	375	10.361	116.7	0.013	1.36	1.362	0.057	5.537	0.00	0.025	5.562	1.000	0.057	5.619	5.592	5.185	5.245	4.810	4.870	0.579	116.7	183.4	133.4
1/G 1/H	1/H1/G	Class 4 RRJ	375	27.592	107.2	0.013	1.27	1.271	0.048	5.592	0.00	0.057	5.649	1.000	0.048	5.697	5.702	5.275	5.415	4.900	5.040	0.507	107.2	171.2	124.9
1/H 1/I	1/I1/H	Class 4 RRJ	375	13.630	96.1	0.013	1.25	1.252	0.039	5.697	0.00	0.023	5.720	1.197	0.046	5.766	5.996	5.445	5.515	5.070	5.140	0.514	96.1	172.2	125.6
1/I 1/J	1/J1/J	Class 2 RRJ	375	20.639	78.5	0.013	1.22	1.217	0.026	5.766	0.00	0.024	5.790	2.074	0.053	5.844	6.357	5.595	5.705	5.220	5.330	0.533	78.5	175.6	128.0
1/J 1/K	1/K1/J	Class 2 RRJ	375	15.424	19.8	0.013	0.83	0.830	0.002	5.844	0.00	0.001	5.845	1.000	0.002	5.847	6.312	5.745	5.825	5.370	5.450	0.519	19.8	173.1	126.3
1/K 1/L	1/L1/K	PVC	225	13.107	10.9	0.009	1.10	1.100	0.004	5.847	0.00	0.005	5.851	14.148	0.054	5.906	6.378	5.705	5.805	5.480	5.580	0.763	10.9	56.2	56.7
1/E 2/A	2/A1/E	Class 4 RRJ	375	7.877	52.3	0.013	1.08	1.082	0.011	5.524	0.00	0.004	5.528	1.940	0.022	5.550	5.554	5.095	5.135	4.720	4.760	0.508	52.3	171.2	124.9
2/A 2/B	2/B2/A	Class 3 RRJ	375	37.800	19.3	0.013	0.82	0.816	0.002	5.550	0.00	0.003	5.554	1.000	0.002	5.558	5.707	5.165	5.355	4.790	4.980	0.503	19.3	170.3	124.3
2/B 2/C	2/C2/B	Class 2 RRJ	300	5.835	14.9	0.013	0.78	0.784	0.002	5.555	0.00	0.001	5.556	1.000	0.002	5.558	5.707	5.330	5.360	5.030	5.060	0.514	14.9	95.8	69.3
2/C 2/D	2/D2/C	Class 2 RRJ	300	11.309	15.2	0.013	0.80	0.797	0.002	5.558	0.00	0.002	5.560	2.105	0.005	5.565	5.948	5.390	5.450	5.090	5.150	0.531	15.2	97.4	70.4
2/D 2/E	2/E2/D	PVC	150	14.101	9.4	0.009	1.12	1.115	0.014	5.565	0.00	0.028	5.594	1.924	0.028	5.621	6.088	5.390	5.508	5.240	5.358	0.835	9.4	20.2	20.1
2/E 2/F	2/F2/E	PVC	150	17.025	3.5	0.009	0.86	0.858	0.002	5.621	0.00	0.006	5.627	14.820	0.029	5.657	6.163	5.508	5.650	5.358	5.500	0.835	3.5	20.2	20.1
1/B 3/A	3/A1/B	Class 2 RRJ	375	7.269	283.7	0.013	1.50	1.501	0.084	5.064	0.00	0.026	5.090	1.000	0.084	5.174	5.257	4.785	4.835	4.410	4.460	0.688	283.7	401.1	290.8
3/A 3/B	3/B3/A	Class 2 RRJ	375	21.725	226.5	0.013	1.33	1.329	0.054	5.174	0.00	0.050	5.224	1.043	0.056	5.280	5.418	4.865	4.985	4.490	4.610	0.552	226.5	357.8	260.6
3/B 3/C	3/C3/B	PVC	300	38.643	151.4	0.009	1.50	1.497	0.059	5.280	0.00	0.123	5.403	1.000	0.059	5.462	5.621	4.940	5.120	4.640	4.820	0.466	151.4	184.9	190.7
3/C 3/D	3/D3/C	PVC	300	25.061	108.1	0.009	1.80	1.798	0.119	5.462	0.01	0.157	5.619	1.907	0.228	5.846	5.772	5.150	5.310	4.850	5.010	0.638	108.1	111.6	111.6
3/D 3/E	3/E3/D	PVC	300	30.941	48.5	0.009	1.59	1.589	0.024	5.772	0.00	0.043	5.815	1.634	0.039	5.855	5.935	5.350	5.570	5.015	5.270	0.711	48.5	115.5	117.8
3/E 3/F	3/F3/E	PVC	300	26.801	25.9	0.009	1.36	1.359	0.007	5.855	0.00	0.012	5.866	2.200	0.015	5.881	6.075	5.600	5.800	5.300	5.500	0.746	25.9	118.5	120.7
3/F 3/G	3/G3/F	Class 2 RRJ	225	10.114	16.1	0.013	0.90	0.903	0.008	5.88															

N.	DESCRIPTION	
C	REVISED ARCHITECTURAL PLANS	
D	DRAFT DA ISSUE	DRN A.C. APP A.C. DATE 06.07.18
T	REVISED TO SUIT CONSULTANTS COMMENTS	A.C. A.C. 16.12.18
M	REVISED TO PLANNERS COMMENTS	A.C. A.C. 19.12.18
G	REVISED TO COUNCIL COMMENTS	A.C. A.C. 25.01.19 M.D. a.c. 16.08.19



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Height Datum	A.H.D.	Client Title	----
Drawn	A.C.	ANGLICARE (ACA), SYDNEY	
Designed	A.C.		
Checked	A.C.		
Approved	A.C.	Dwg Status APPROVAL	Local Authority BANKSTOWN CITY COUNCIL

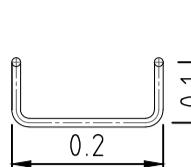
Hydrologic Design Sheet -Drainage Building C							
Return Period: 10 Location: Milperra							
Pit	Catchment Flow Length	Catchment Slope	Catchment Roughness	Catchment Flow Time	Gutter Flow Time	Total Time	Pit Type
	(m)	(%)	'n'	(min)	(min)		
12/B							
12/C							
12/D							
13/A							
13/B							
13/C							
13/D							
13/E							
14/B							
14/C							
14/D							
14/E							
14/F							
14/G							
14/H							
14/I							
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14/Y							
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15/H							
15/I							
15/J							
15/K							
15/L							
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15/N							
15/O							
15/P							
15/Q							
15/R							
15/S							
15/T							
15/U							
15/V							
15/W							
15/X							
15/Y							
15/Z							
16/A							
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16/J							
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16/M							
16/N							
16/O							
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17/W							
17/X							
17/Y							
17/Z							
18/A							
18/B							

PROPOSED ANGLICAN RETIREMENT VILLAGE			Ref & Dwg No
BULLECOURT AVE, MILPERRA			16116.DA.C16
DRAINAGE CALCULATIONS			Sheet No
			16 of 27
			Scale
			AS SHOWN
			Date
			25.01.19
			Rev
			G
			A1

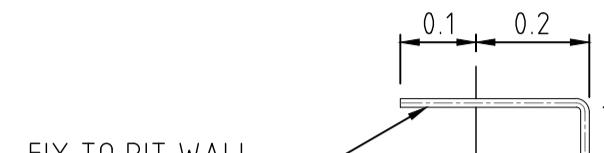
Pit Schedule - Proposed												
Pit No.	Pit Type	Pit Type	Pit Width	Pit Length	Outlet Diameter	Outlet Invert RL	Inlet Diameter	Inlet Invert RL	Pit Depth	Pit Lid Level	Eastings	Northings
			(mm)	(mm)	(mm)	(m)	(mm)	(m)	(m)	(m)		
1/A	Headwall	525	1600	780	375	4.360	0.327	4.687	313853.22	6243062.40		
1/B	GSIP	2700 x 900	900	2700	375	4.370	375	4.400	0.694	5.064	313854.28	6243061.04
1/C	GSIP	1800 x 900	900	1800	375	4.430	225	4.510	0.879	5.309	313860.16	6243061.34
1/D	GSIP	1800 x 900	900	1800	375	4.540	375	4.570	0.882	5.422	313871.67	6243070.53
1/E	GSIP	1800 x 900	900	1800	375	4.690	375	4.720	0.905	5.595	313894.81	6243067.26
1/F	GSIP	900 x 900	900	900	375	4.780	375	4.810	0.757	5.537	313900.94	6243058.09
1/G	KIP 2	1% on grade	900	900	375	4.870	375	4.900	0.722	5.592	313911.22	6243056.81
1/H	KIP 2	1% on grade	900	900	375	5.040	375	5.070	0.662	5.702	313938.55	6243052.99
1/I	KIP 2	1% on grade	900	900	375	5.140	375	5.220	0.856	5.996	313947.61	6243042.80
1/J	GSIP	900 x 900	900	900	375	5.330	375	5.370	1.027	6.357	313947.76	6243022.16
1/K	KIP 2	1% on grade	900	900	375	5.450	225	5.480	0.862	6.312	313945.54	6243006.90
1/L	KIP 2	1% on grade	900	900	225	5.580			0.798	6.378	313943.71	6242993.92
2/A	KIP 2	1% on grade	900	900	375	4.760	375	4.790	0.794	5.554	313901.90	6243063.83
2/B	KIP 2	1% on grade	900	900	375	4.980	300	5.030	0.722	5.702	313939.34	6243058.63
2/C	KIP 2	1% on grade	900	900	300	5.060	300	5.090	0.647	5.707	313944.76	6243060.78
2/D	KIP 2	1% on grade	900	900	300	5.150	150	5.240	0.798	5.948	313956.01	6243059.56
2/E	KIP 2	1% on grade	900	900	150	5.358	150	5.358	0.731	6.088	313957.94	6243073.53
2/F	GSIP	900 x 900	900	900	150	5.500			0.663	6.163	313959.69	6243090.47
3/A	GSIP	1800 x 900	900	1800	375	4.460	225	4.490	0.797	5.257	313851.48	6243054.33
3/B	GSIP	1800 x 900	900	1800	375	4.610	300	4.640	0.808	5.418	313841.61	6243034.98
3/C	GSIP	1800 x 900	900	1800	300	4.820	300	4.850	0.801	5.621	313846.35	6242996.62
3/D	GSIP	900 x 900	900	900	300	5.010	300	5.050	0.762	5.772	313860.30	6242975.81
3/E	GSIP	900 x 900	900	900	300	5.270	300	5.300	0.665	5.935	313890.47	6242968.92
3/F	GSIP	900 x 900	900	900	300	5.500	225	5.530	0.575	6.075	313917.01	6242965.22
3/G	KIP 2	1% on grade	900	900	225	5.600			0.478	6.078	313984.81	6242975.24
4/A	GSIP	900 x 900	900	900	150	5.100			0.618	5.718	313843.17	6242982.03
5/A	GSIP	900 x 900	900	900	225	4.600			2.146	6.746	313855.47	6243046.82
6/A	GSIP	900 x 900	900	900	225	4.686			2.242	6.928	313878.32	6243064.25

NOTES:

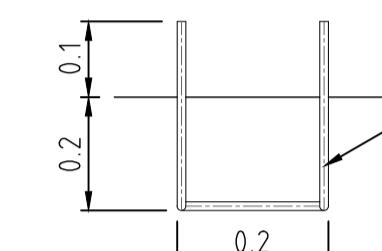
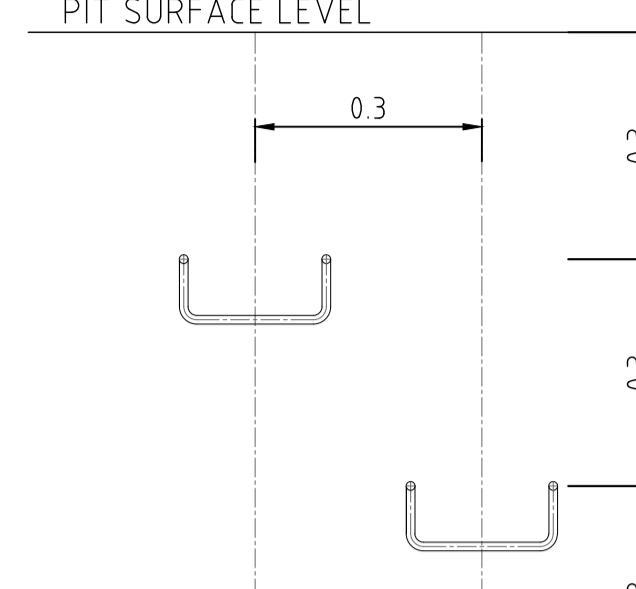
- ALL PITS OVER 1.2m DEEP REQUIRE STEP IRONS TO DETAIL.
- ALL KERB INLET PITS ARE TO BE CONSTRUCTED TO STANDARD DEPARTMENT OF HOUSING DRAWINGS RM10.
- 'GSIP' PITS ARE TO BE GRATED AND 'JP' PITS ARE TO HAVE SEALED LIDS. KIP 1, KIP 2, KIP 3 & KIP 5 ARE TO BE KERB INLET PITS WITH 1.8m, 2.4m, 3.0m AND 4.8m LINTELS RESPECTIVELY.



ELEVATION



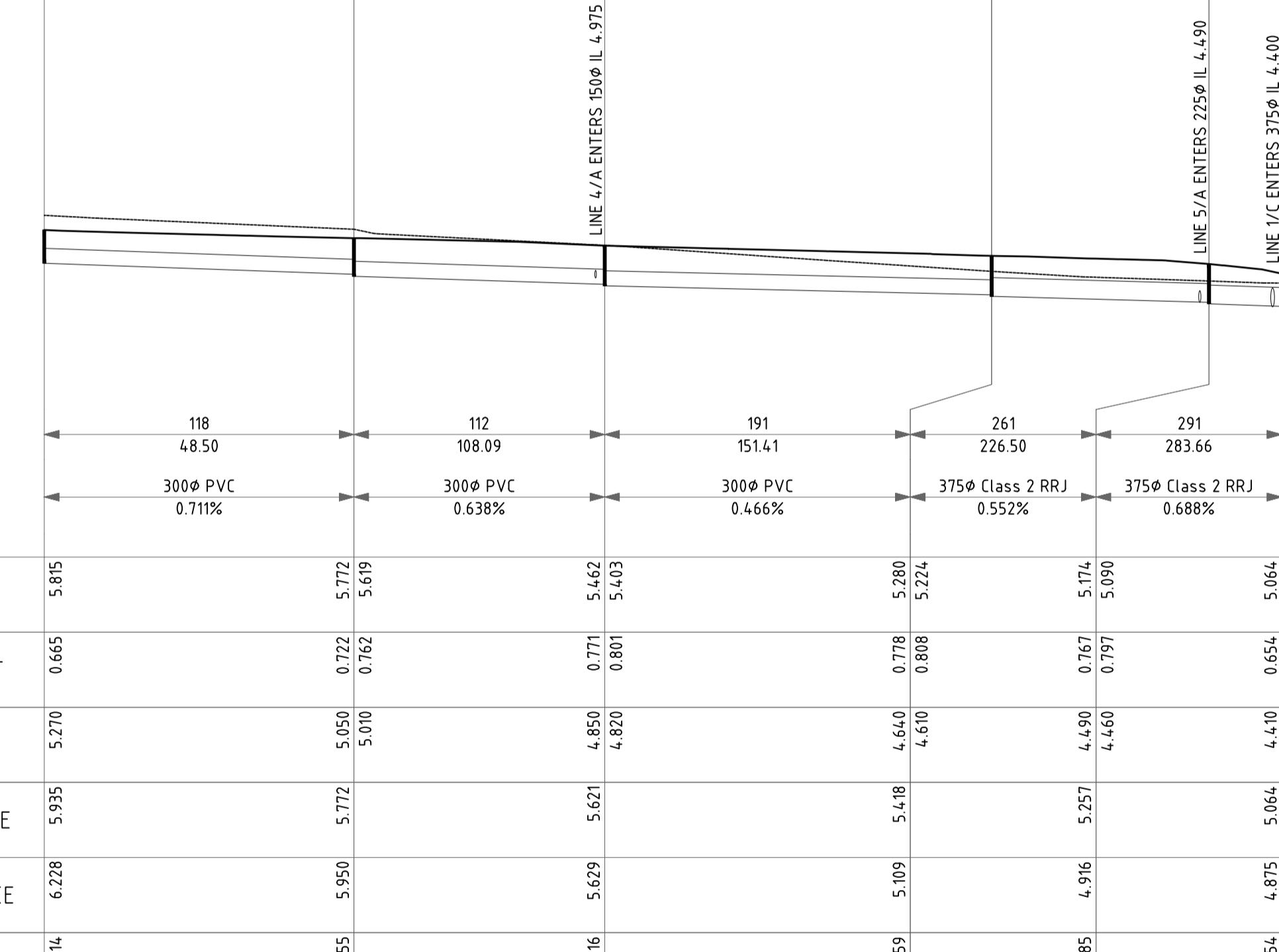
SECTION

STEP IRON DETAIL
SCALE 1:10ALL PITS OVER 1.2m DEEP
TO CONTAIN STEP IRONS

STEP IRON PLACEMENT

11/A	Headwall	225-450	1100	565		300	4.070	0.393	4.463	313811.71	6243113.53	
11/B	GSIP	900 x 900	900	900	300	4.110	300	4.140	0.533	4.643	313817.96	6243112.93
11/C	GSIP	900 x 900	900	900	300	4.250	300	4.280	0.548	4.798	313835.40	6243109.01
11/D	GSIP	900 x 900	900	900	300	4.390			0.572	4.962	313853.90	6243104.72
10/A	KIP 2	SAG	900	900	375	4.870			0.858	5.728	313975.00	6243116.50
7/A	Headwall	225-450	1100	565		375	4.620	0.041	4.661	313949.28	6243130.42	
7/B	KIP 2	1% on grade	900	900	375	4.700	375	4.730	1.067	5.767	313960.60	6243125.81
7/C	KIP 2	SAG	900	900	375	4.790	375	4.820	0.938	5.728	313968.65	6243117.39
7/D	GSIP	900 x 900	900	900	375	4.980	375	5.020	0.841	5.821	313964.39	6243086.91
11/A	Headwall	225-450	1100	565		300	4.070	0.393	4.463	313811.71	6243113.53	
11/B	GSIP	900 x 900	900	900	300	4.110	300	4.140	0.533	4.643	313817.96	6243112.93
11/C	GSIP	900 x 900	900	900	300	4.250	300	4.280	0.548	4.798	313835.40	6243109.01
11/D	GSIP	900 x 900	900	900	300	4.390			0.572	4.962	313853.90	6243104.72
12/A	Headwall	225-450	1100	565		225	4.400	0.204	4.604	313798.84	6243025.22	
12/B	GSIP	1200 x 1200	1200	1200	225	4.430	225	4.560	0.537	4.967	313800.74	6243025.00
12/C	GSIP	900 x 900	900	900	225	4.540	225	4.570	1.624	6.164	313806.20	6243010.93
12/D	GSIP	900 x 900	900	900	225	4.870			1.667	6.537	313816.89	6243009.43
13/A	GSIP	900 x 900	900	900	225	4.620	225	4.650	2.037	6.657	313816.63	6243023.48
13/B	GSIP	900 x 900	900	900	225	4.850			1.896	6.746	313814.09	6243041.12
15/A	Headwall	225-450	1100	565		3						

CAPACITY (l/sec)
FLOW (l/sec)
PIPE DETAILS
GRADE
DATUM RL -0.6
HGL
DEPTH TO INVERT
INVERT LEVEL
FINISHED SURFACE
EXISTING SURFACE
CHAINAGE



LINE 3/A ENTERS 150φ IL 6.975

LINE 4/A ENTERS 150φ IL 6.975

LINE 5/A ENTERS 225φ IL 4.490

LINE 6/A ENTERS 375φ IL 4.490

LINE 7/A ENTERS 375φ IL 5.260

LINE 8/A ENTERS 375φ IL 5.260

LINE 9/A ENTERS 375φ IL 5.020

LINE 10/A ENTERS 375φ IL 4.830

LINE 11/A ENTERS 375φ IL 4.830

LINE 12/A ENTERS 375φ IL 4.830

LINE 13/A ENTERS 375φ IL 4.830

LINE 14/A ENTERS 375φ IL 4.830

LINE 15/A ENTERS 375φ IL 4.830

LINE 16/A ENTERS 375φ IL 4.830

LINE 17/A ENTERS 375φ IL 4.830

LINE 18/A ENTERS 375φ IL 4.830

LINE 19/A ENTERS 375φ IL 4.830

LINE 20/A ENTERS 375φ IL 4.830

LINE 21/A ENTERS 375φ IL 4.830

LINE 22/A ENTERS 375φ IL 4.830

LINE 23/A ENTERS 375φ IL 4.830

LINE 24/A ENTERS 375φ IL 4.830

LINE 25/A ENTERS 375φ IL 4.830

LINE 26/A ENTERS 375φ IL 4.830

LINE 27/A ENTERS 375φ IL 4.830

LINE 28/A ENTERS 375φ IL 4.830

LINE 29/A ENTERS 375φ IL 4.830

LINE 30/A ENTERS 375φ IL 4.830

LINE 31/A ENTERS 375φ IL 4.830

LINE 32/A ENTERS 375φ IL 4.830

LINE 33/A ENTERS 375φ IL 4.830

LINE 34/A ENTERS 375φ IL 4.830

SCALES: HORIZ 1:500 VERT 1:100

LINE 3
SCALES: HORIZ 1:500 VERT 1:100

LINE 4

SCALES: HORIZ 1:500 VERT 1:100

LINE 5

SCALES: HORIZ 1:500 VERT 1:100

LINE 6

SCALES: HORIZ 1:500 VERT 1:100

LINE 7

SCALES: HORIZ 1:500 VERT 1:100

LINE 8

SCALES: HORIZ 1:500 VERT 1:100

LINE 9

SCALES: HORIZ 1:500 VERT 1:100

LINE 10

SCALES: HORIZ 1:500 VERT 1:100

LINE 11

SCALES: HORIZ 1:500 VERT 1:100

LINE 12

SCALES: HORIZ 1:500 VERT 1:100

LINE 13

SCALES: HORIZ 1:500 VERT 1:100

LINE 14

SCALES: HORIZ 1:500 VERT 1:100

LINE 15

SCALES: HORIZ 1:500 VERT 1:100

LINE 16

SCALES: HORIZ 1:500 VERT 1:100

LINE 17

SCALES: HORIZ 1:500 VERT 1:100

LINE 18

SCALES: HORIZ 1:500 VERT 1:100

LINE 19

SCALES: HORIZ 1:500 VERT 1:100

LINE 20

SCALES: HORIZ 1:500 VERT 1:100

LINE 21

SCALES: HORIZ 1:500 VERT 1:100

LINE 22

SCALES: HORIZ 1:500 VERT 1:100

LINE 23

SCALES: HORIZ 1:500 VERT 1:100

LINE 24

SCALES: HORIZ 1:500 VERT 1:100

LINE 25

SCALES: HORIZ 1:500 VERT 1:100

LINE 26

SCALES: HORIZ 1:500 VERT 1:100

LINE 27

SCALES: HORIZ 1:500 VERT 1:100

LINE 28

SCALES: HORIZ 1:500 VERT 1:100

LINE 29

SCALES: HORIZ 1:500 VERT 1:100

LINE 30

SCALES: HORIZ 1:500 VERT 1:100

LINE 31

SCALES: HORIZ 1:500 VERT 1:100

LINE 32

SCALES: HORIZ 1:500 VERT 1:100

LINE 33

SCALES: HORIZ 1:500 VERT 1:100

- REvised Architectural Plans
- Draft DA Issue
- Revised to Suit Consultants Comments
- Revised to Planners Comments
- Revised to Council Comments

N.	C.	Z.	DESCRIPTION
G	T	M	REVISED ARCHITECTURAL PLANS
D	A	I	DRAFT DA ISSUE
C	R	S	REVISED TO SUIT CONSULTANTS COMMENTS
A	P	O	REVISED TO PLANNERS COMMENTS
M	E	C	REVISED TO COUNCIL COMMENTS

CAPACITY (l/sec)		FLOW (l/sec)		GRADE	
HGL		0.699	5.606	RL 0.3	
DEPTH TO INVERT		0.699	5.606	134	15.00
INVERT LEVEL		0.699	5.606	134	14.15
FINISHED SURFACE		5.260	5.260	134	14.15
EXISTING SURFACE		5.549	5.549	138	15.26
CHAINAGE		6.833	6.834	138	15.26
		6.141	6.072	138	15.26
		5.999	5.991	138	15.26
		5.300	5.300	138	15.26
		0.699	0.699	138	15.26

LINE 8

SCALES: HORIZ 1:500 VERT 1:100

LINE 9

SCALES: HORIZ 1:500 VERT 1:100

LINE 10

SCALES: HORIZ 1:500 VERT 1:100

LINE 11

SCALES: HORIZ 1:500 VERT 1:100

LINE 12

SCALES: HORIZ 1:500 VERT 1:100

LINE 13

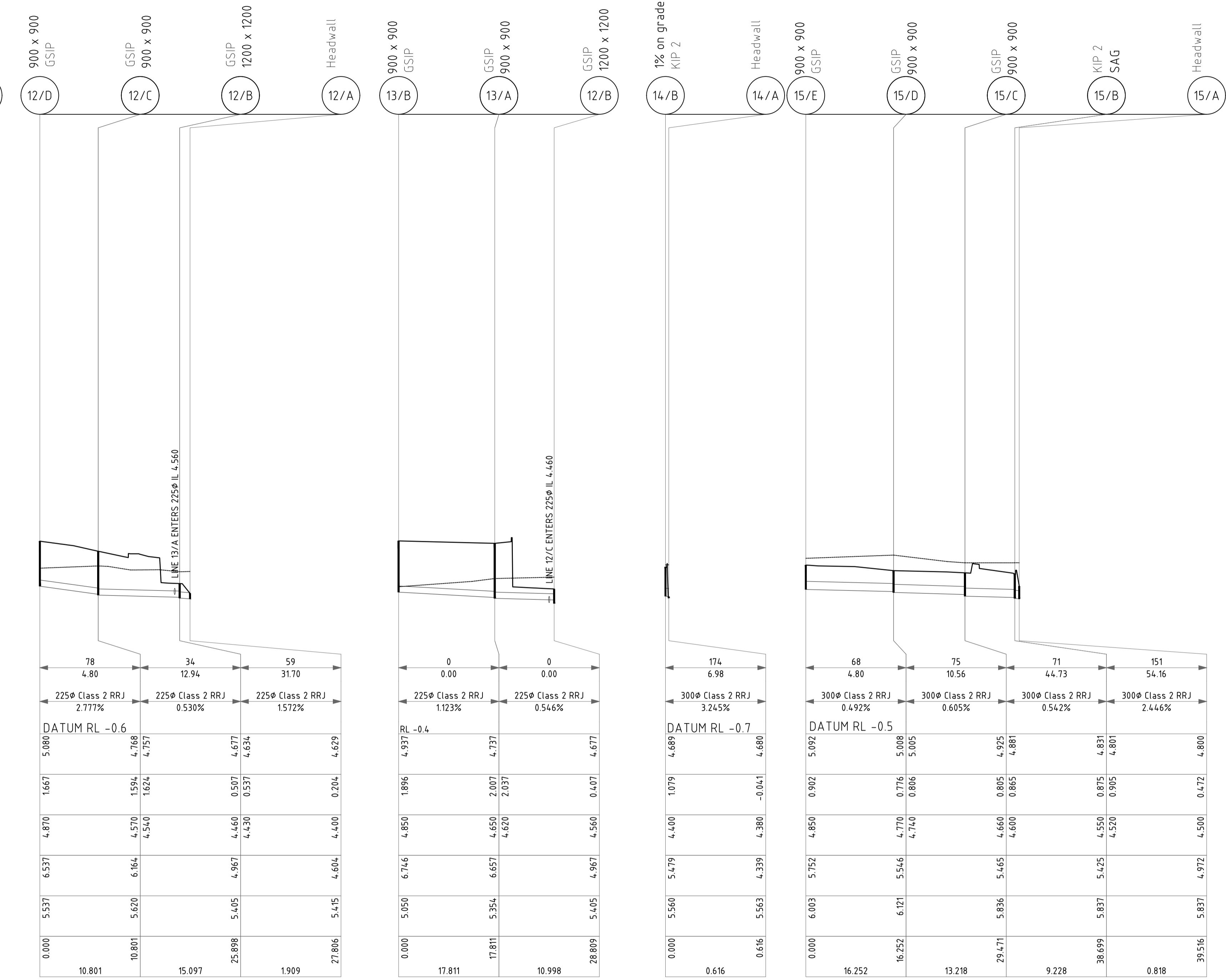
SCALES: HORIZ 1:500 VERT 1:100

LINE 14

SCALES: HORIZ 1:500 VERT 1:100

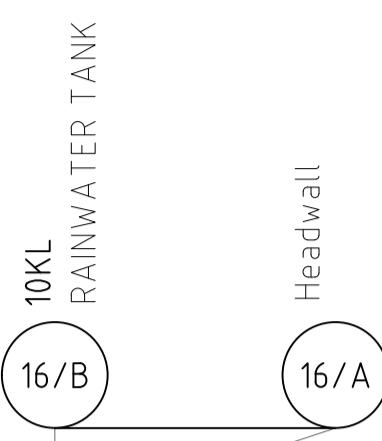
LINE 15

SCALES: HORIZ 1:500 VERT 1:100



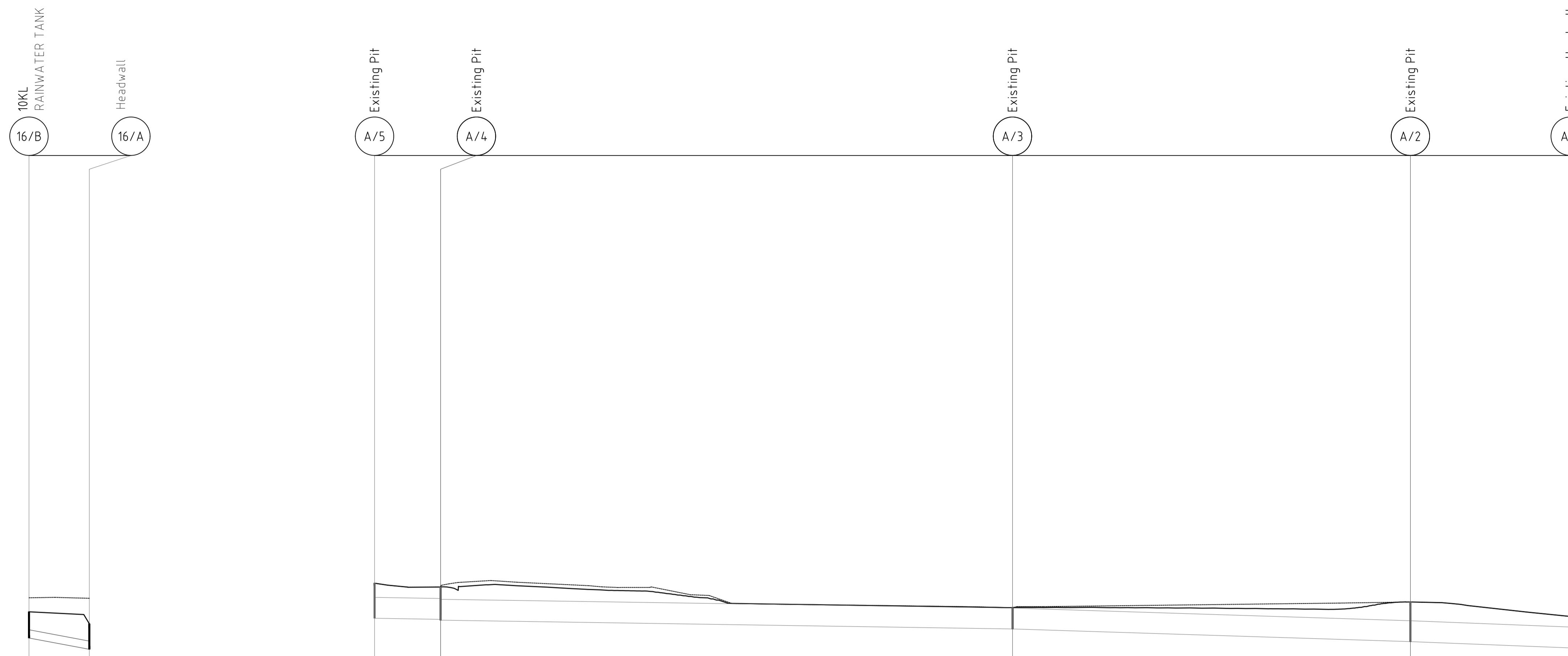
	CAPACITY (l/sec)	FLOW (l/sec)	PIPE DETAILS	GRADE	DATUM RL -1.1
HGL					
DEPTH TO INVERT	5.235	4.281	0.954	4.94	
INVERT LEVEL					
FINISHED SURFACE	5.745	4.600	3.869	0.931	4.169
EXISTING SURFACE	5.726	4.600	3.869	0.931	4.169
CHAINAGE	0.000	10.983	10.983		

LINE 16
SCALES: HORIZ 1:500 VERT 1:100



	CAPACITY (l/sec)	FLOW (l/sec)	PIPE DETAILS	GRADE	DATUM RL -0.4
HGL					
DEPTH TO INVERT	6.982	5.700	1.286	9.946	
INVERT LEVEL					
FINISHED SURFACE	6.336	5.630	1.176	6.836	
EXISTING SURFACE	6.369	5.630	1.206	8.738	
CHAINAGE	0.000	12.022	12.022		

EXISTING Ø750 LINE
SCALES: HORIZ 1:500 VERT 1:100



N	T	M	D	C	DESCRIPTION
G	T	M	D	C	REVISED ARCHITECTURAL PLANS
F	R	I	S	A	DRAFT DA ISSUE
G	T	M	D	C	REVISED TO SUIT CONSULTANTS COMMENTS
F	R	I	S	A	REVISED TO PLANNERS COMMENTS
G	T	M	D	C	REVISED TO COUNCIL COMMENTS



HEAD OFFICE
345 Keira St
Wollongong NSW 2500
PO Box 1044
Wollongong NSW 2520
T 61 2 4227 4233
F 61 2 4227 4133
E info@siteplus.com.au

Height Datum
A.H.D.

Drawn
A.C.

Designed
A.C.

Checked
A.C.

Approved
A.C.

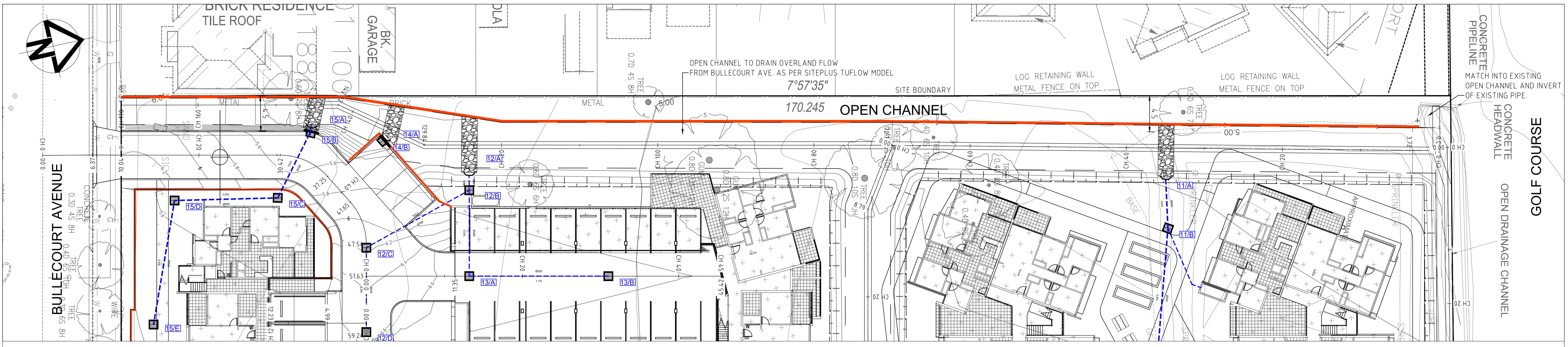
Client Title

ANGLICARE (ACA), SYDNEY

Dwg Title
PROPOSED ANGLICAN RETIREMENT VILLAGE
BULLECOURT AVE, MILPERRA
DRAINAGE LONGSECTIONS

Ref & Dwg No
16116.DA.C21
Sheet No
21 of 27
Scale
AS SHOWN
Date
25.01.19 Rev
G A1

Dwg Status APPROVAL Local Authority BANKSTOWN CITY COUNCIL



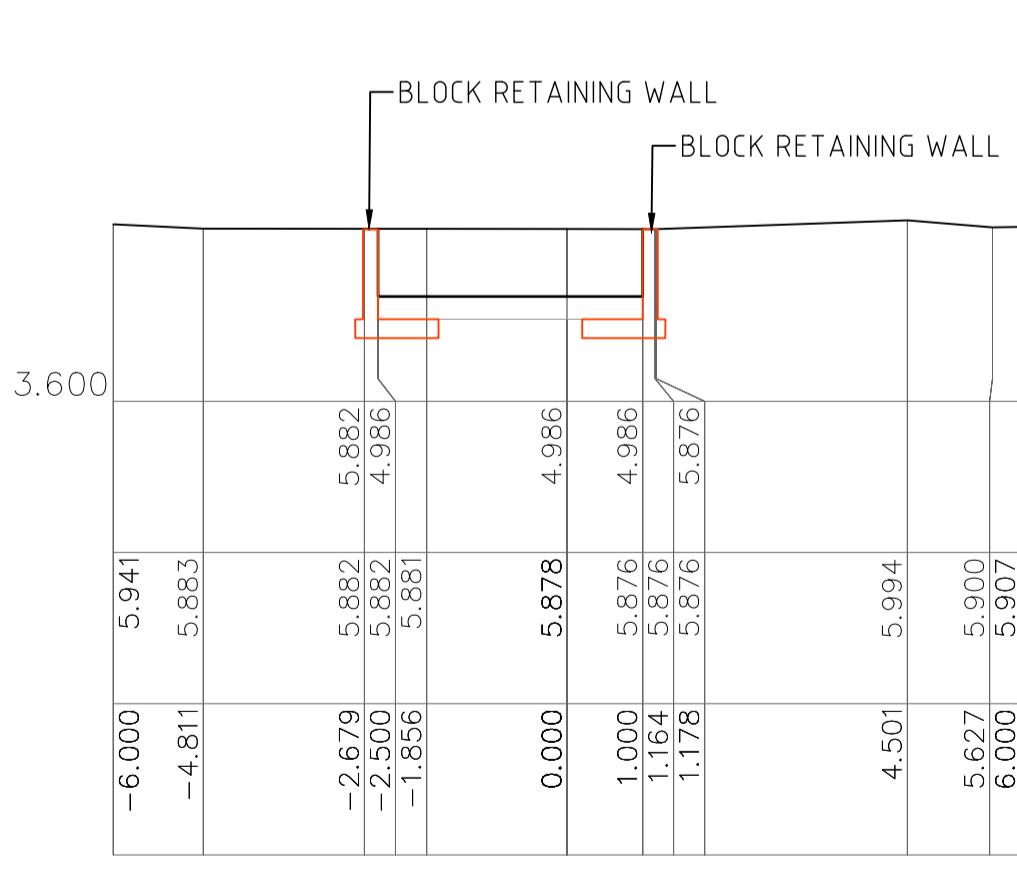
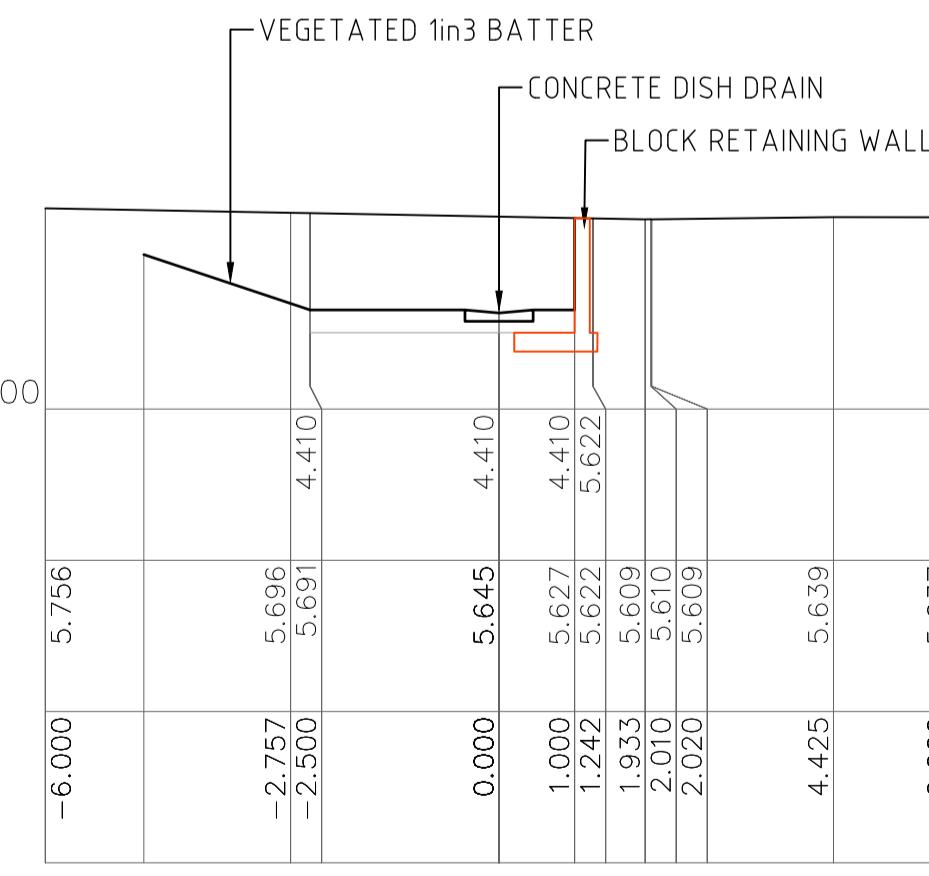
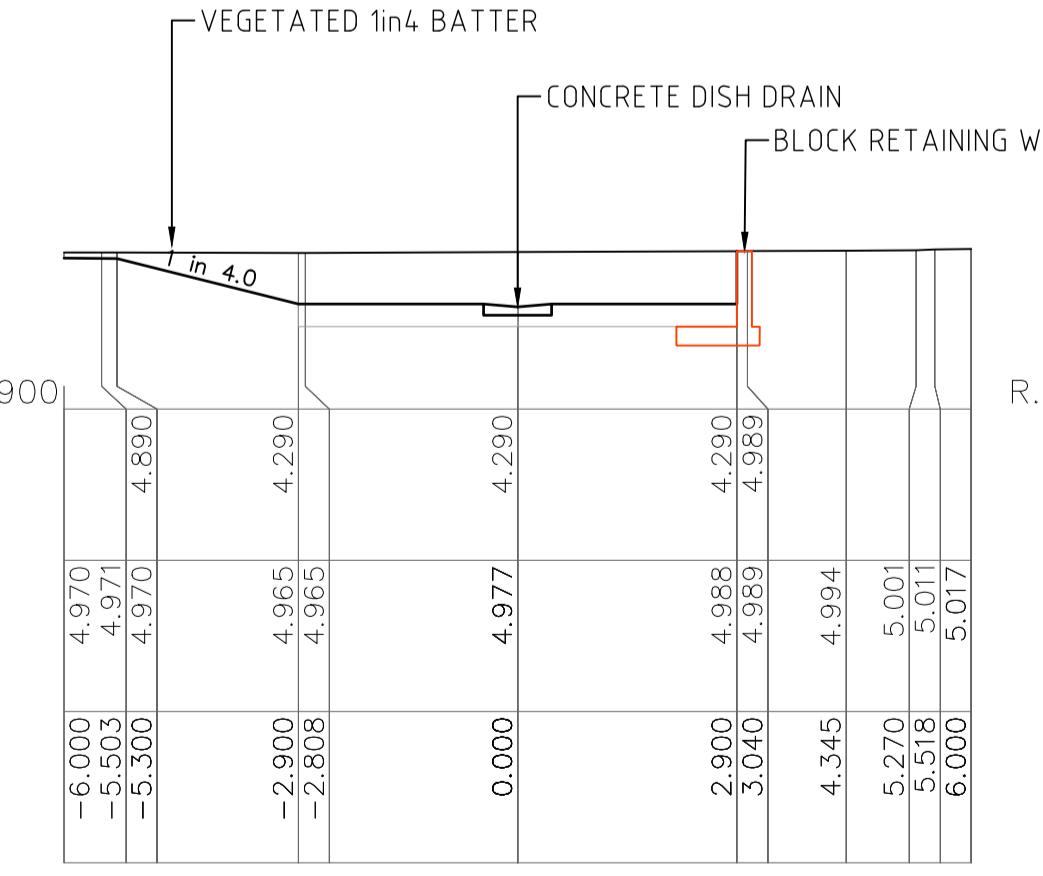
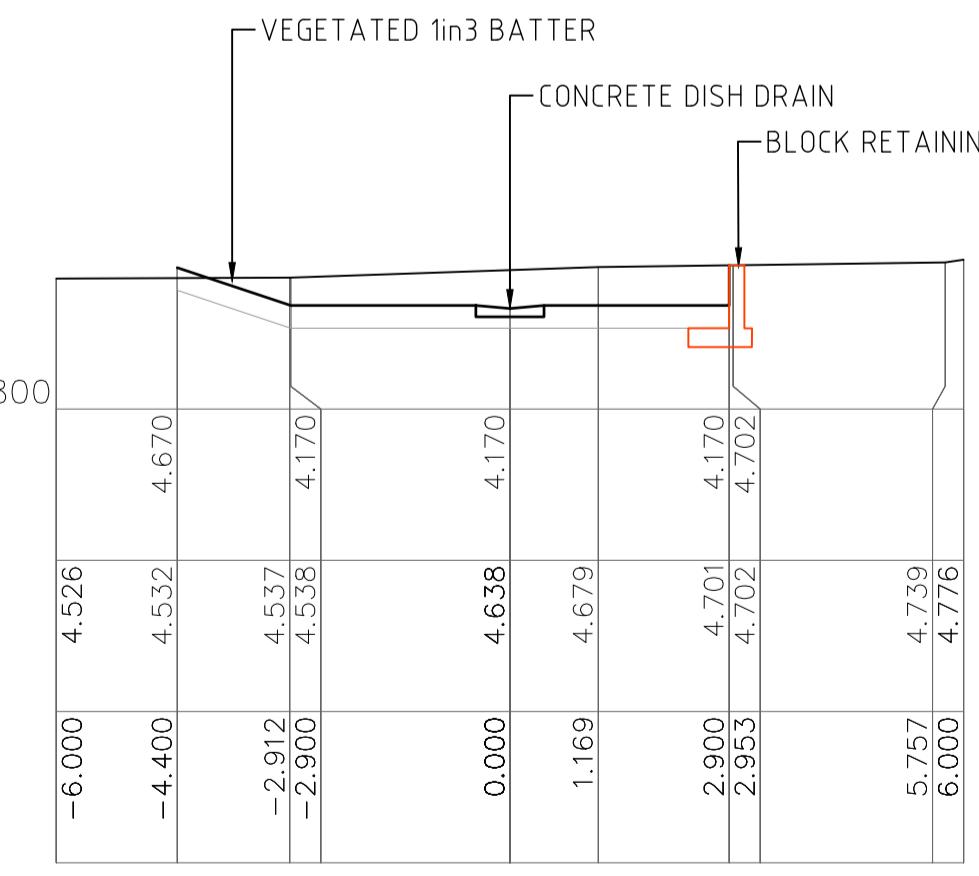
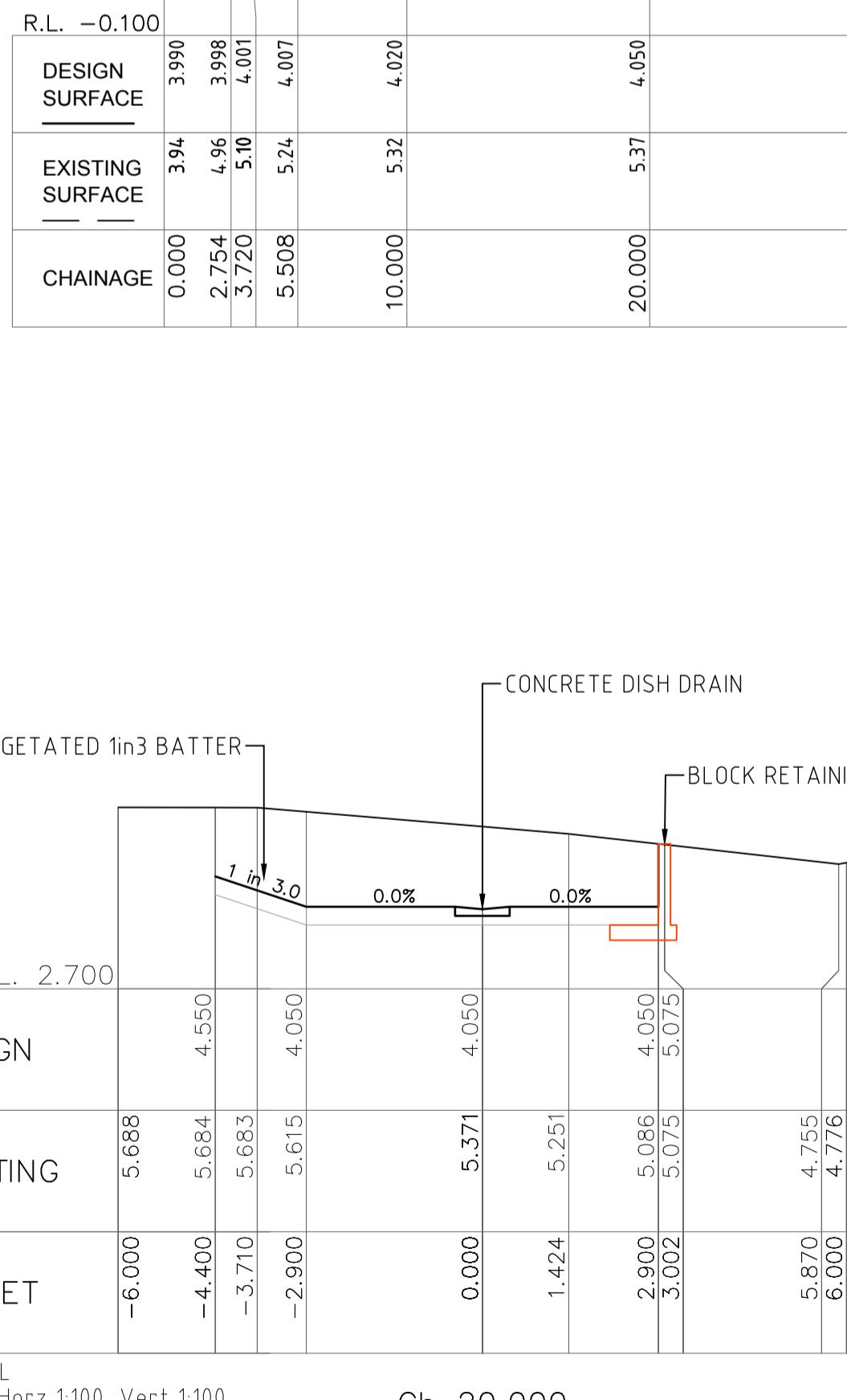
CHANNEL

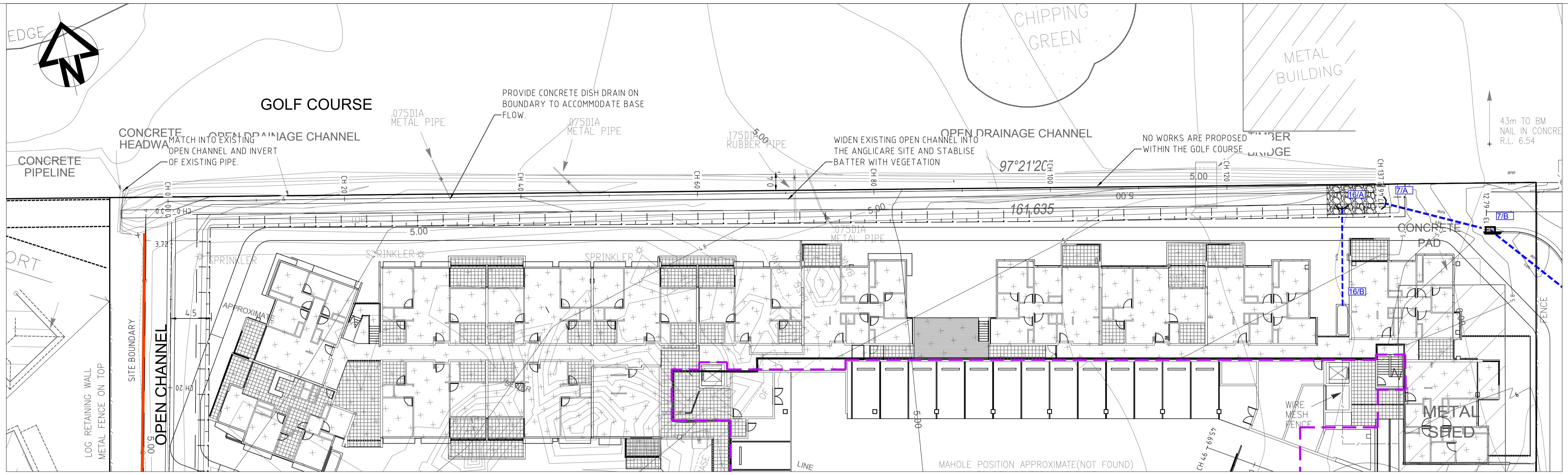
SCALE 1:250

Meters

SCALE : - 1 : 250 @ A1
- 1 : 500 @ A3

LONGITUDINAL SECTION
CHANNEL Ch 0.000 to Ch 169.850
SCALES: HORIZONTAL 1:250 VERTICAL 1:100

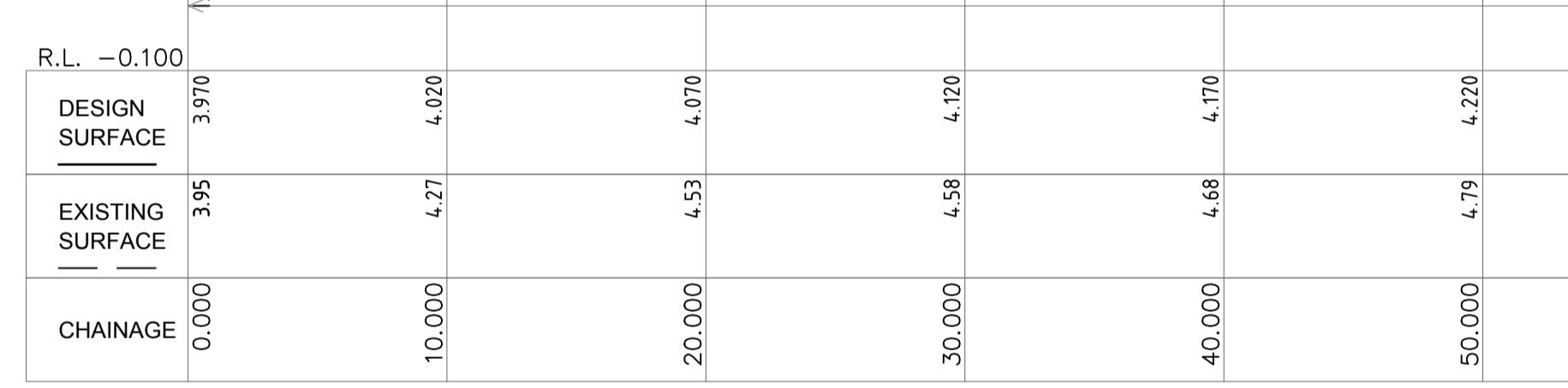




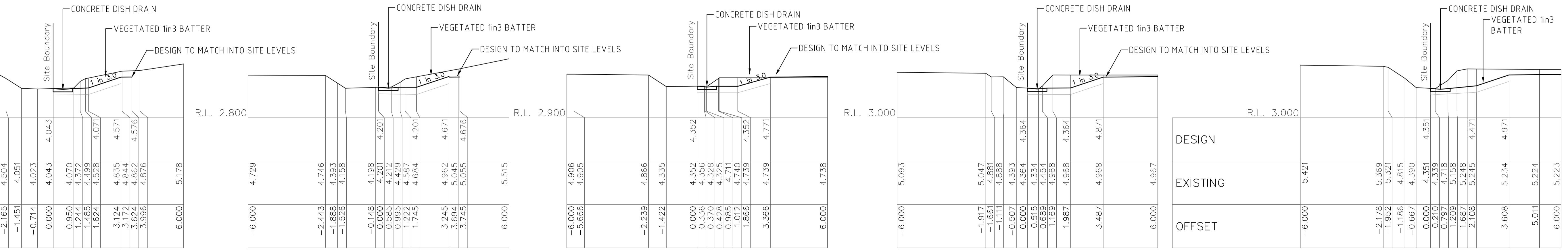
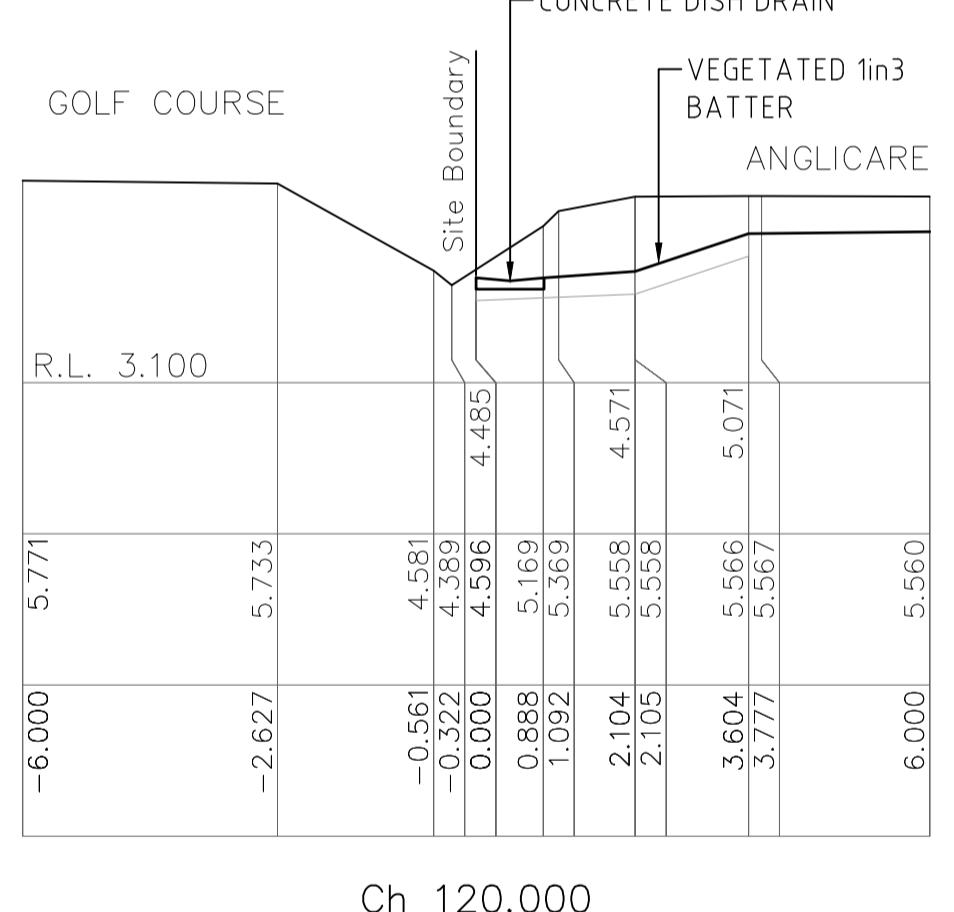
NORTHERN BDY CHANNEL

SCALE 1:250
0 5 10 15 20 25
Meters

SCALE : 1 : 250 @ A1
- 1 : 500 @ A3



LONGITUDINAL SECTION
NORTHERN BDY CHANNEL BOTTOM Ch 0.000 to Ch 138.100
SCALES: HORIZONTAL 1:250 VERTICAL 1:100



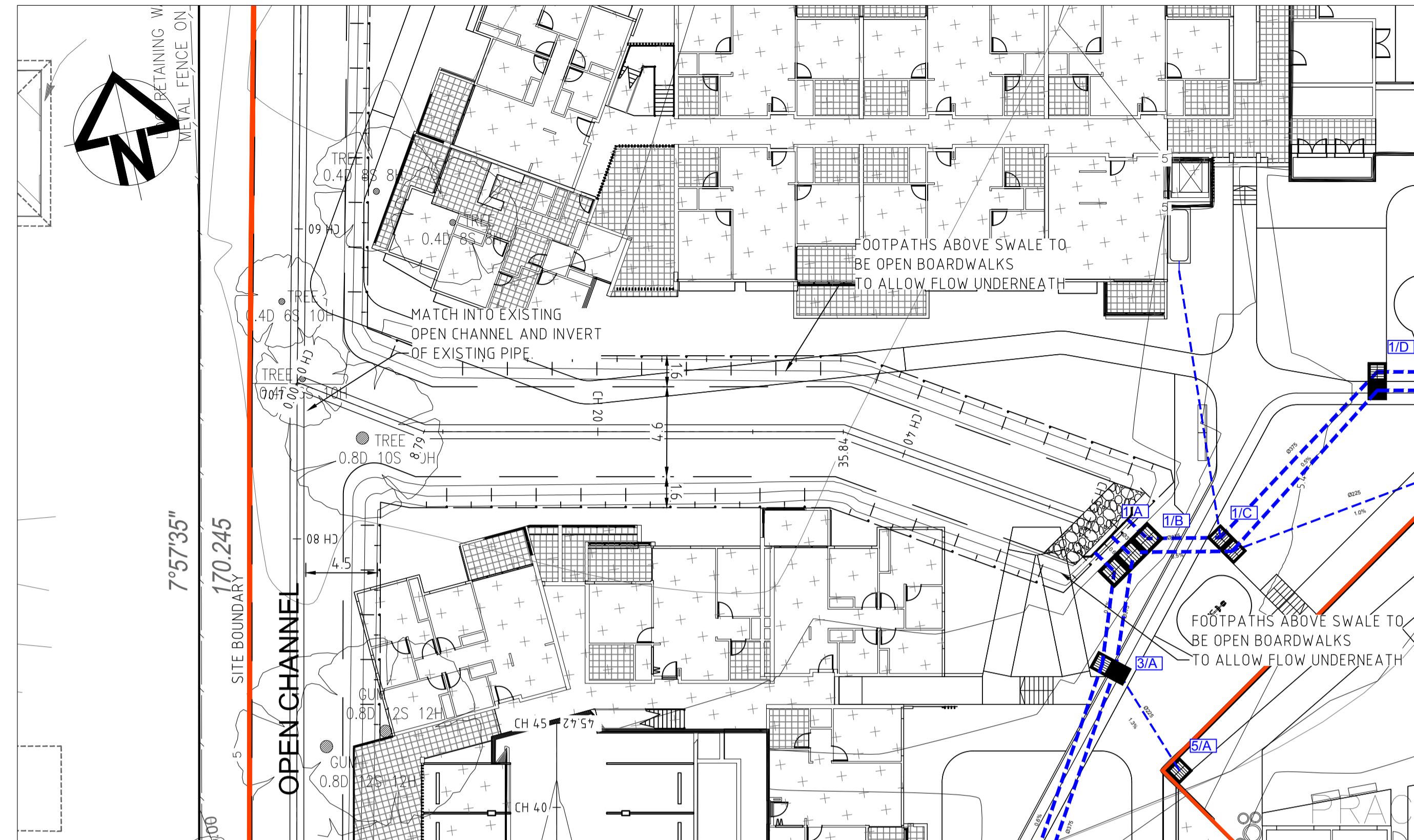
No.	C	Z.	DESCRIPTION
G	T	M	REVISED ARCHITECTURAL PLANS
D	A	C	DRAFT DA ISSUE
C	A	C	REVISED TO SUIT CONSULTANTS COMMENTS
M	A	C	REVISED TO PLANNERS COMMENTS
G	A	C	REVISED TO COUNCIL COMMENTS

REVISIONS
G T M D C Z.

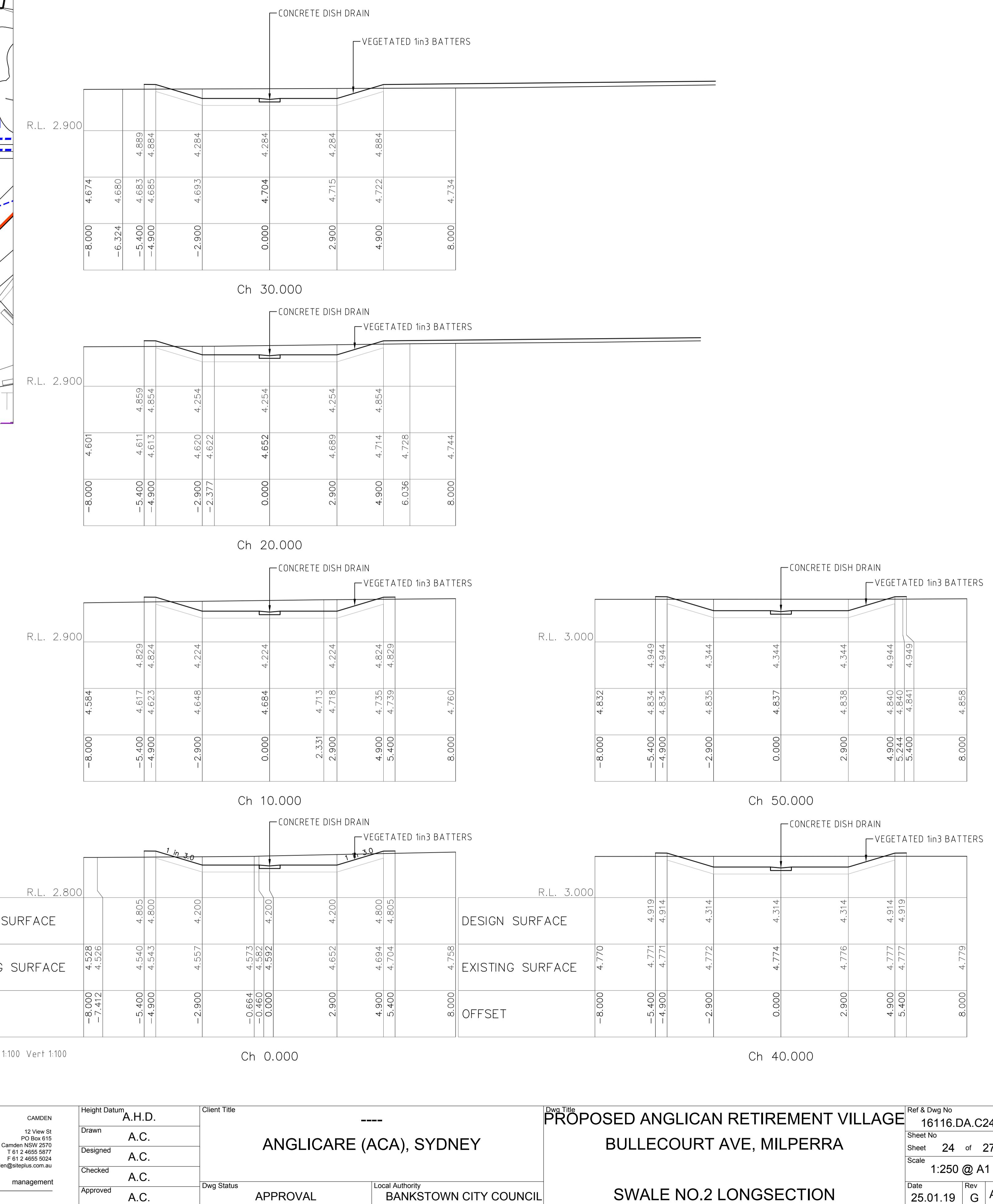
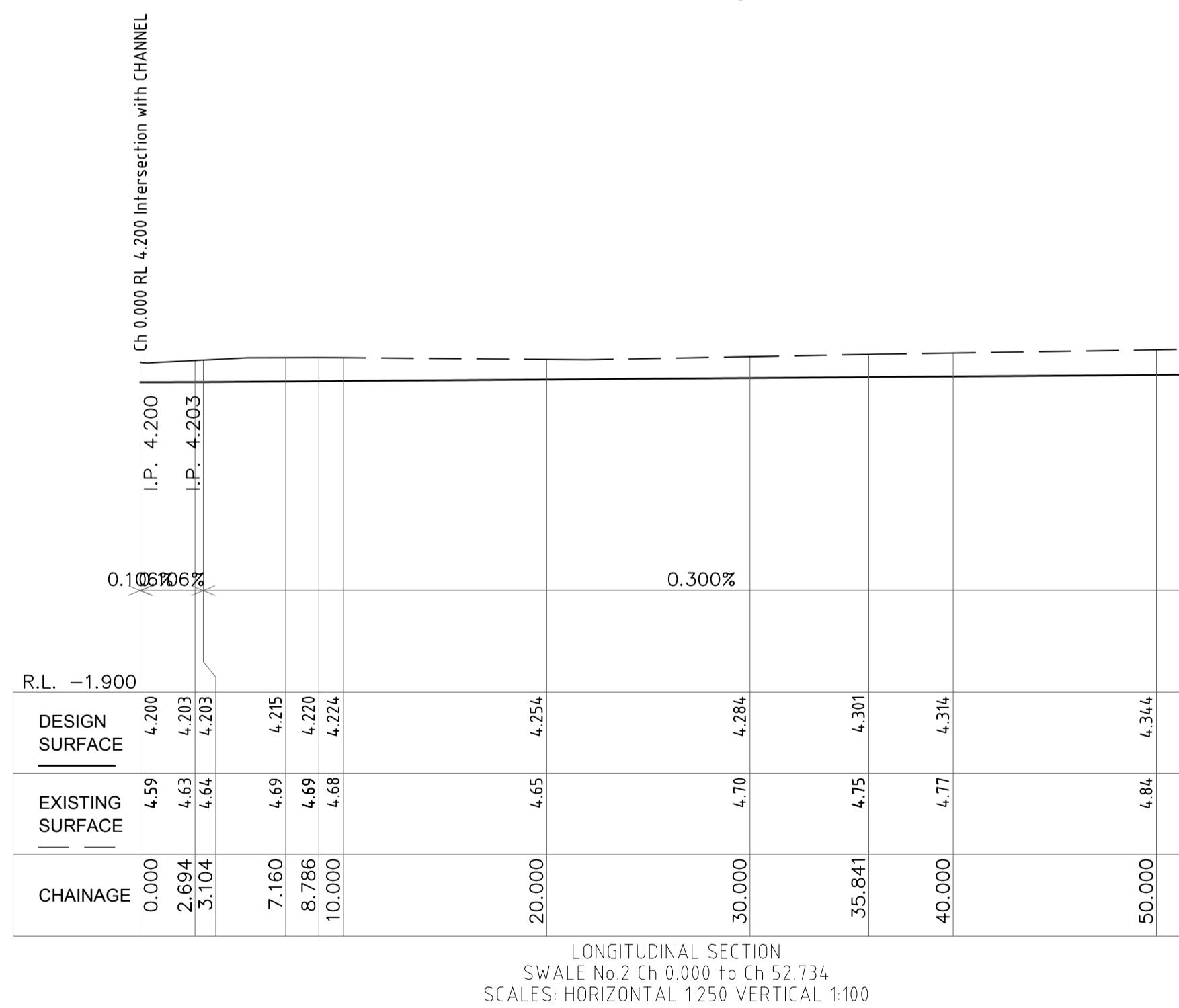
DESCRIPTION
REVISED ARCHITECTURAL PLANS
DRAFT DA ISSUE
REVISED TO SUIT CONSULTANTS COMMENTS
REVISED TO PLANNERS COMMENTS
REVISED TO COUNCIL COMMENTS

DRN APP DATE
A.C. A.C. 06.07.18
A.C. A.C. 16.12.18
A.C. A.C. 19.12.18
A.C. A.C. 25.01.19
M.D. a.c. 16.08.19

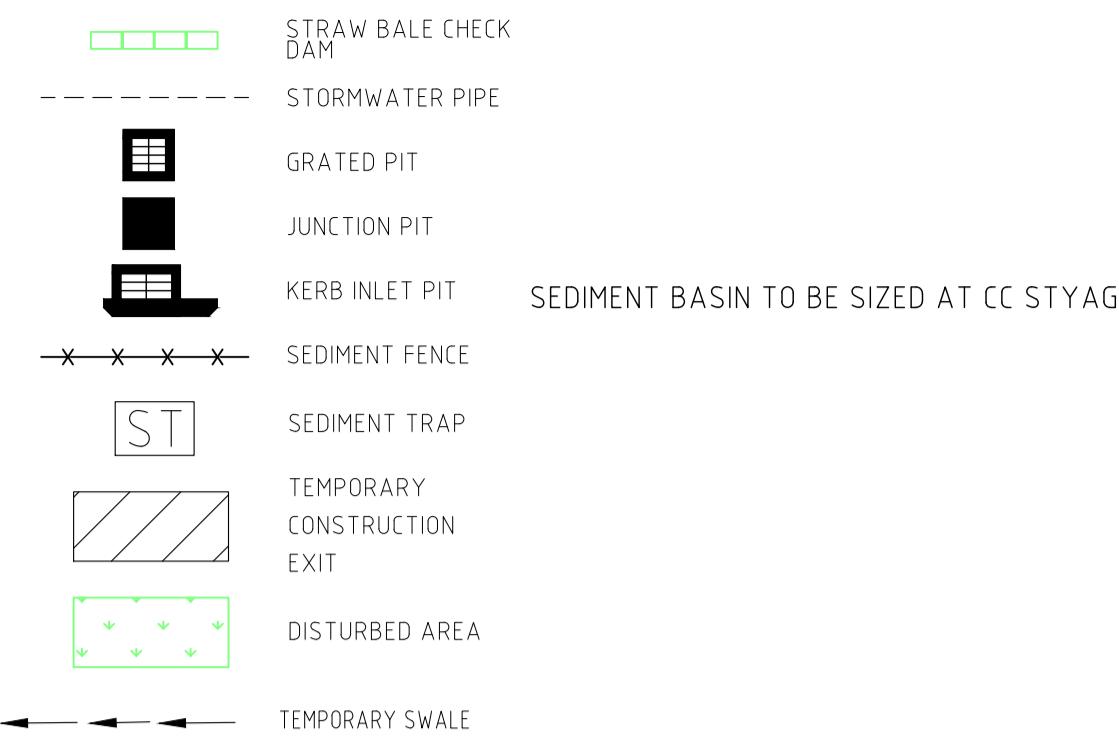
siteplus
Site Plus Pty Ltd ABN 73 104 315 095
planning engineering landscape design management



SWALE No.2
SCALE 1:250
0 5 10 15 20 25
Meters
SCALE : - 1 : 250 @ A1
- 1 : 500 @ A3



LEGEND PROPOSED



MAINTENANCE PROCEDURES DURING CONSTRUCTION

- ALL EROSION CONTROL MEASURES ARE TO BE MAINTAINED AT ALL TIMES SO THAT THOSE MEASURES ARE FULLY FUNCTIONAL / OPERATIONAL DURING THE CURRENCY OF WORKS. ALL SUCH CONTROLS MUST ALSO BE FULLY FUNCTIONAL / OPERATIONAL SHOULD WORK OPERATIONS CEASE TEMPORARILY, (e.g. WEEKENDS, ROSTERED DAYS OFF, etc.)
- RESREAD MATERIAL GAINED DURING MAINTENANCE OPERATION OR ALTERNATIVELY PLACE ON STOCKPILES.

STATEMENT OF SOIL MANAGEMENT

- ALL TOPSOIL IS TO BE STOCKPILED IN AREAS DESIGNATED ON PLAN.
- ALL FORMED EMBANKMENTS (CUT & FILL) ARE TO BE SEDED WITHIN 7 DAYS.
- ALL DISTURBED AREAS INCLUDING ANY CONTROLLED FILL ARE TO BE TOPSOILED & SEDED PRIOR TO COMPLETION OF WORKS. ALL DISTURBED AREAS THAT WILL NOT BE STABILIZED WITHIN 2 MONTHS MUST BE TEMPORARILY REVEGETATED WITHIN 7 DAYS OF CLEARING AREAS THAT FAIL TO ESTABLISH MUST BE RESOWN IMMEDIATELY.
- ALL GULLY PITS ARE TO BE PROVIDED WITH SEDIMENT FILTER BARRIERS SUCH AS STRAW BALE OR SANDBAGS.
- PROVIDE TEMPORARY DIVERSION BANKS ON SITE AS DIRECTED.
- AT THE TOP OF ALL FILL EMBANKMENTS, PROVIDE A BERM TO PREVENT WATER RUNNING DOWN EMBANKMENTS. BERM IS TO BE MAINTAINED AT ALL TIMES AS THE FILL EMBANKMENT PROGRESSES TO FINAL LEVEL.
- ALL FORMED EMBANKMENTS (CUT & FILL) TO BE TOPSOILED & SEDED/FERTILIZED ON FINAL TRIMMING.
- FOR TEMPORARY REVEGETATION PURPOSES, THE REVEGETATION MIXTURE SHOULD BE COUNCIL SPECIFICATIONS.
- TRENCHES FOR INTERLOT DRAINAGE LINES ARE TO BE REINSTATED WITH TOPSOIL FOLLOWING PIPE INSTALLATION & BACKFILLING & IMMEDIATELY SEDED/FERTILIZED.
- THE GROUND SHALL BE TYNED / SCARIFIED TO A MIN DEPTH 100mm PRIOR TO SEEDING.
- CONTRACTOR SHOULD LIASE REGULARLY WITH ENGINEER TO DETERMINE AREAS WHICH CAN BE PROGRESSIVELY REVEGETATED.
- TO ASSIST IN DUST CONTROL/WIND EROSION A TRACKIFIER SUCH AS CUROSOL SHOULD BE USED. OR EQUIVALENT ENDORSED PRODUCT BY EPA, RATES AS PER MANUFACTURES SPECIFICATION.
- DESIGNED PLANT AND MACHINERY ACCESSWAYS TO BE DEFINED ON SITE BY THE INSTALLATION OF PARAWEBBING FENCING TO MINIMIZE UNNECESSARY SITE DISTURBANCE.
- IT IS THE CONTRACTORS RESPONSIBILITY TO MAINTAIN SOIL EROSION CONTROL DEVICES AND CONTROL DUST.

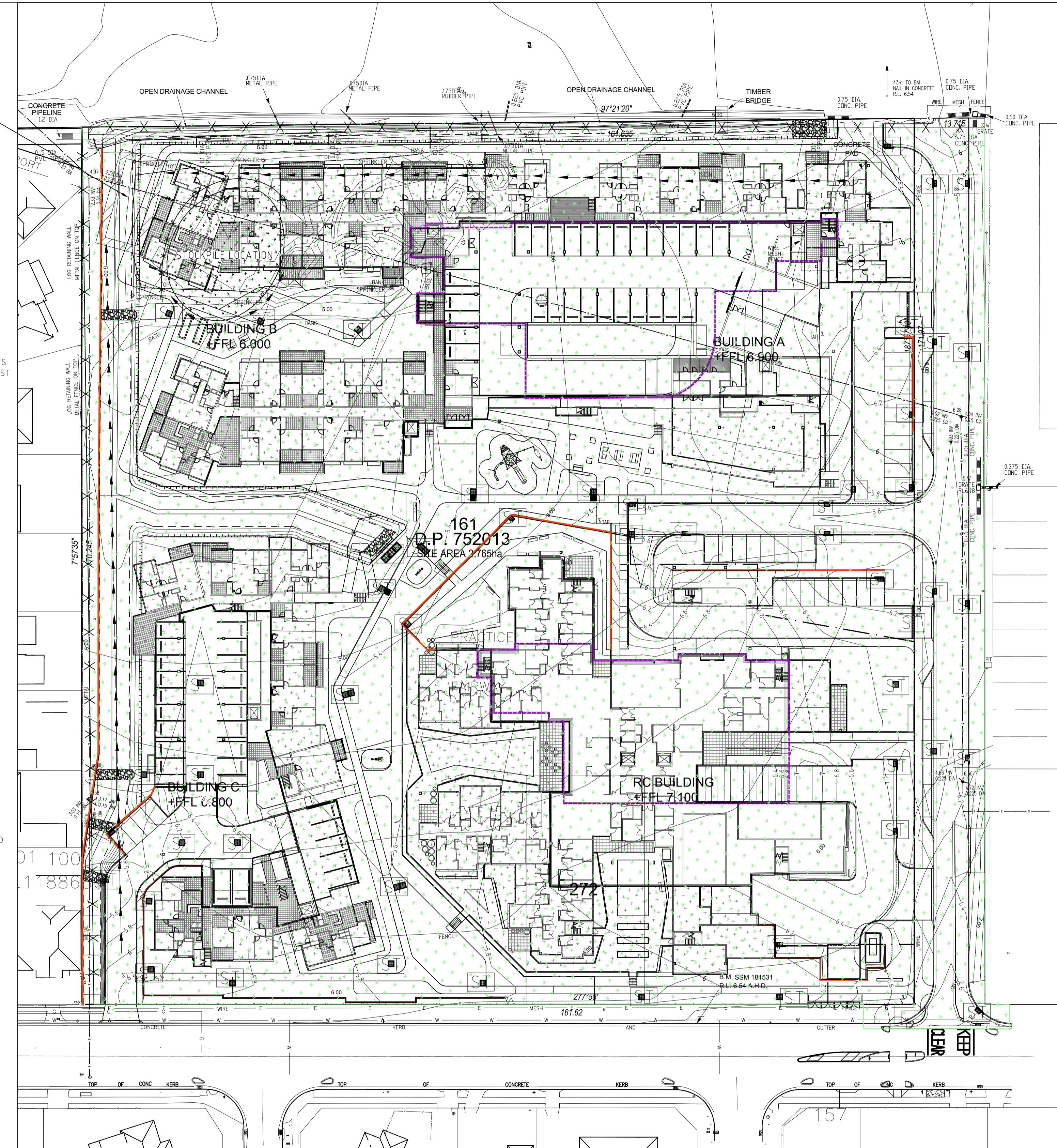
GENERAL NOTES

- REMOVE ALL SOIL & SPOIL MATERIAL FROM THE ROADWAY & GUTTER SYSTEM FOR THAT SECTION OF TRENCHING & / OR OTHER WORKS CARRIED OUT ON ANY ONE DAY. THIS IS TO BE UNDERTAKEN ON THE COMPLETION OF EACH DAY'S WORK.
- ABSTAIN FROM PLACING SPOIL / STOCKPILES ON OR IN THE IMMEDIATE VICINITY OF THE KERB & GUTTER SYSTEM. A MINIMUM SET BACK OF THREE (3) METRES WILL BE REQUIRED.
- STOCKPILES REFERRED TO IN ITEM 2 ARE NOT TO BE PLACED WITHIN FIFTEEN (15) METRES EITHER SIDE OF KERB INLET PITS. THIS IS TO BE REGARDED AS A DESIGNATED EXCLUSION ZONE.
- WHERE ROCK IS ENCOUNTERED DURING THE TRENCHING OPERATIONS & THIS MATERIAL IS CONSIDERED TO BE UNSUITABLE AS BACKFILL, THEN ALL OVERSIZED ROCK WILL BE REQUIRED TO BE REMOVED FROM THE SITE & WILL NOT BE PERMITTED TO REMAIN ON THE SURFACE OF REGRADING & SURFACE PREPARATION PRIOR TO SEEDING DISTURBED AREAS.

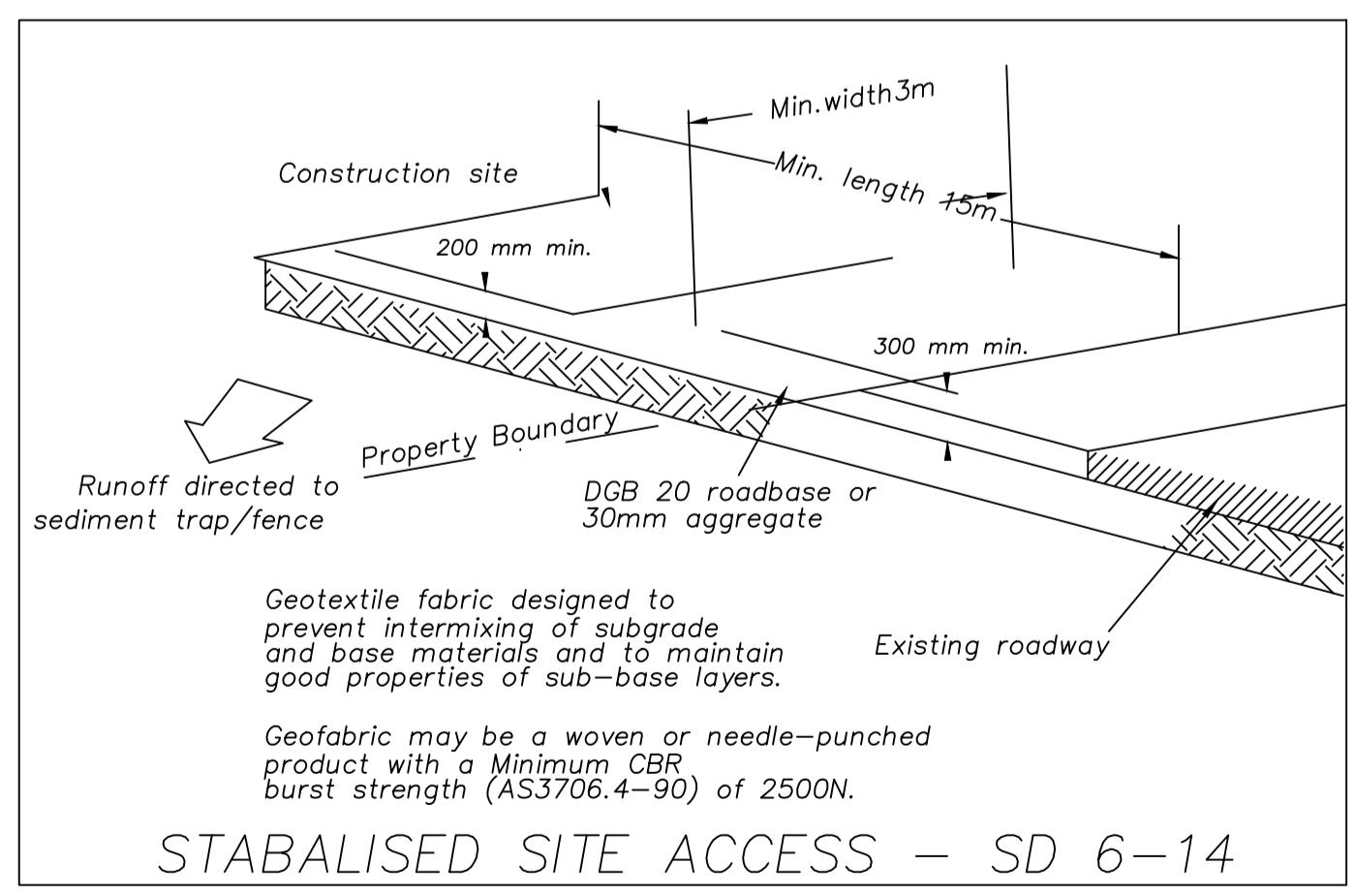
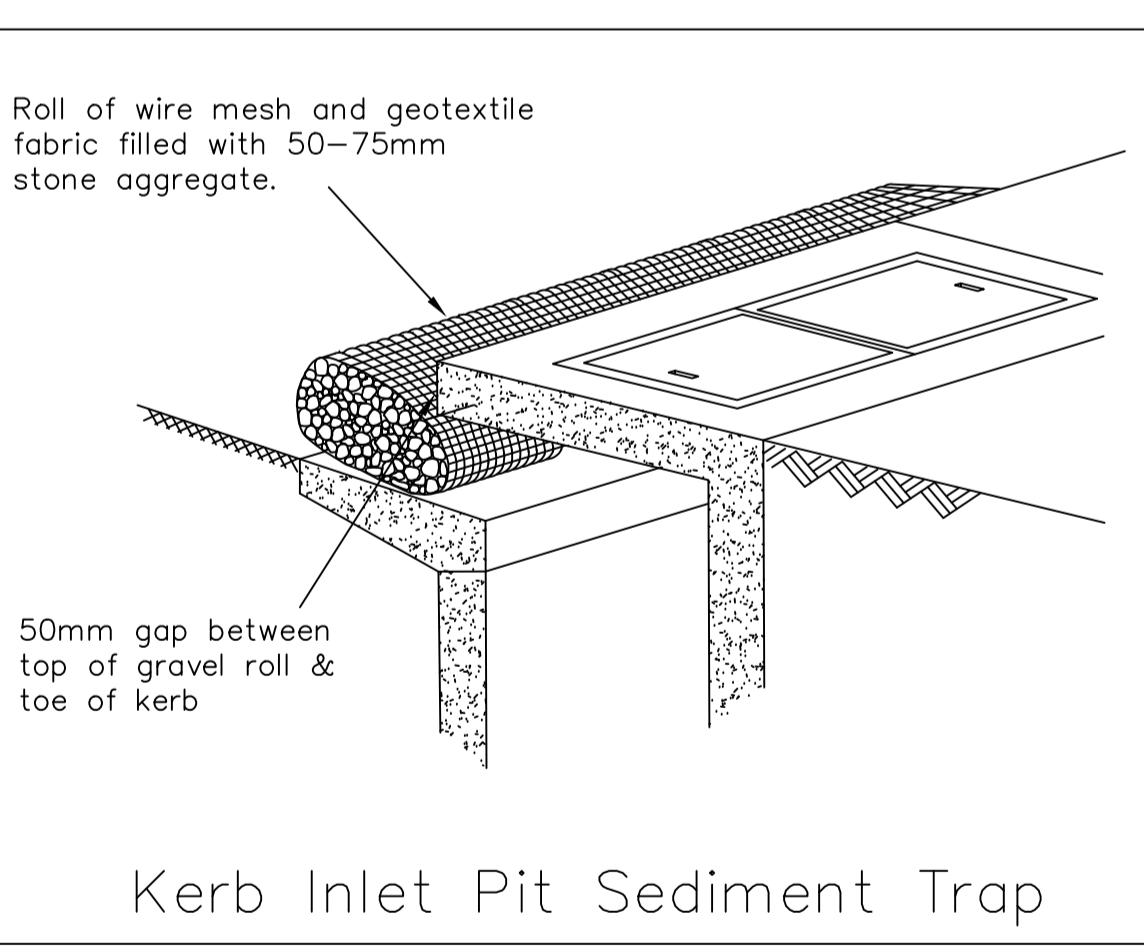
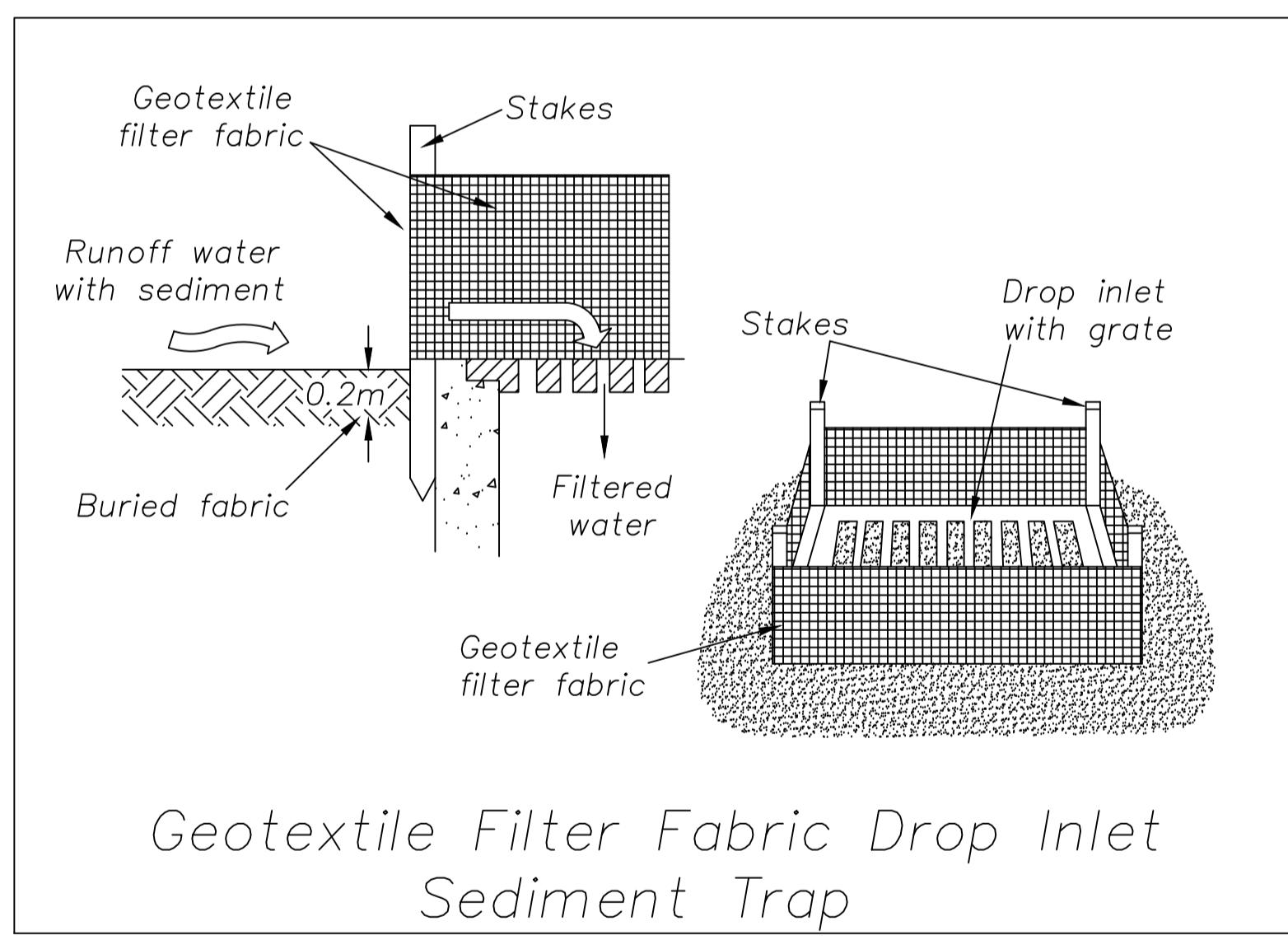
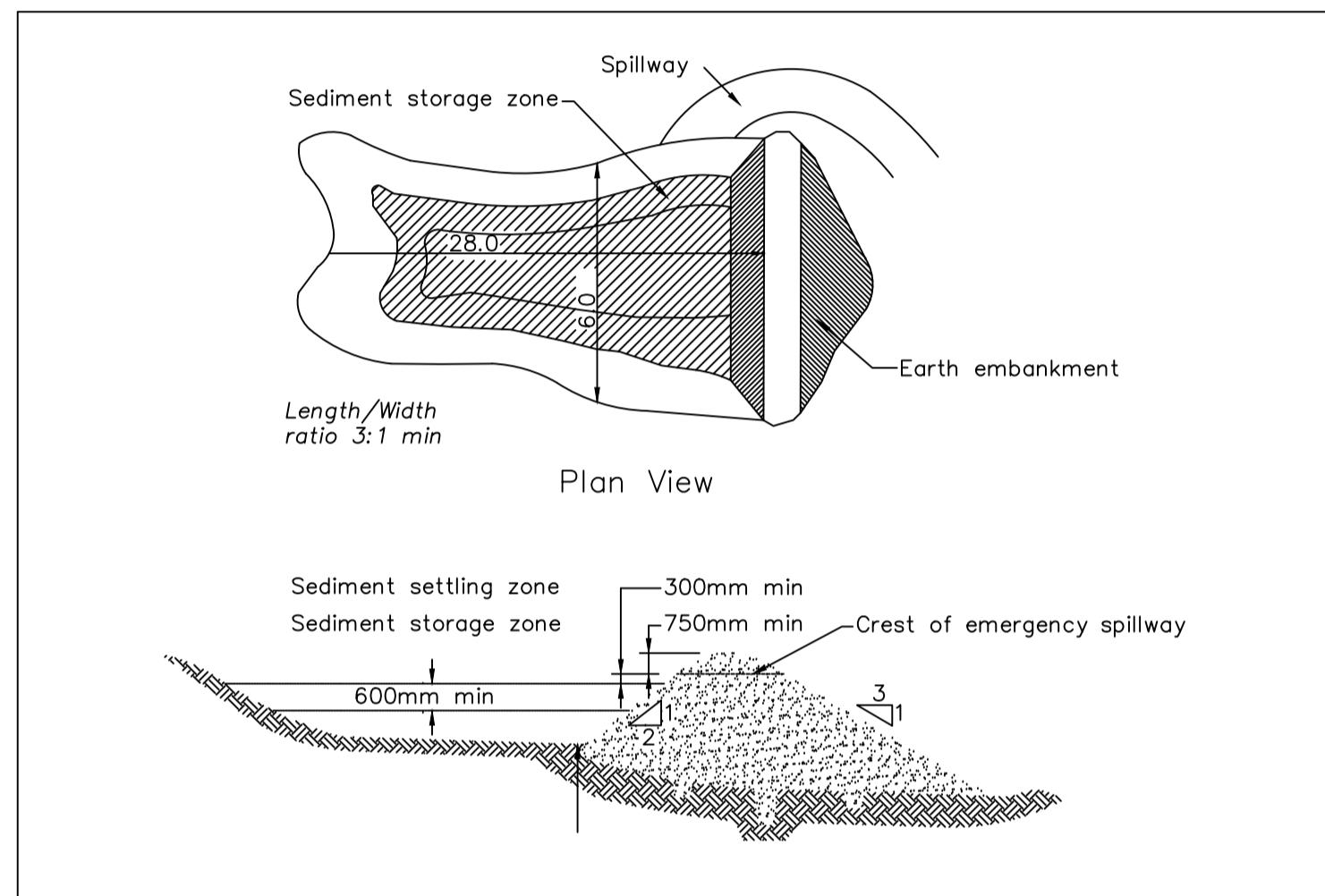
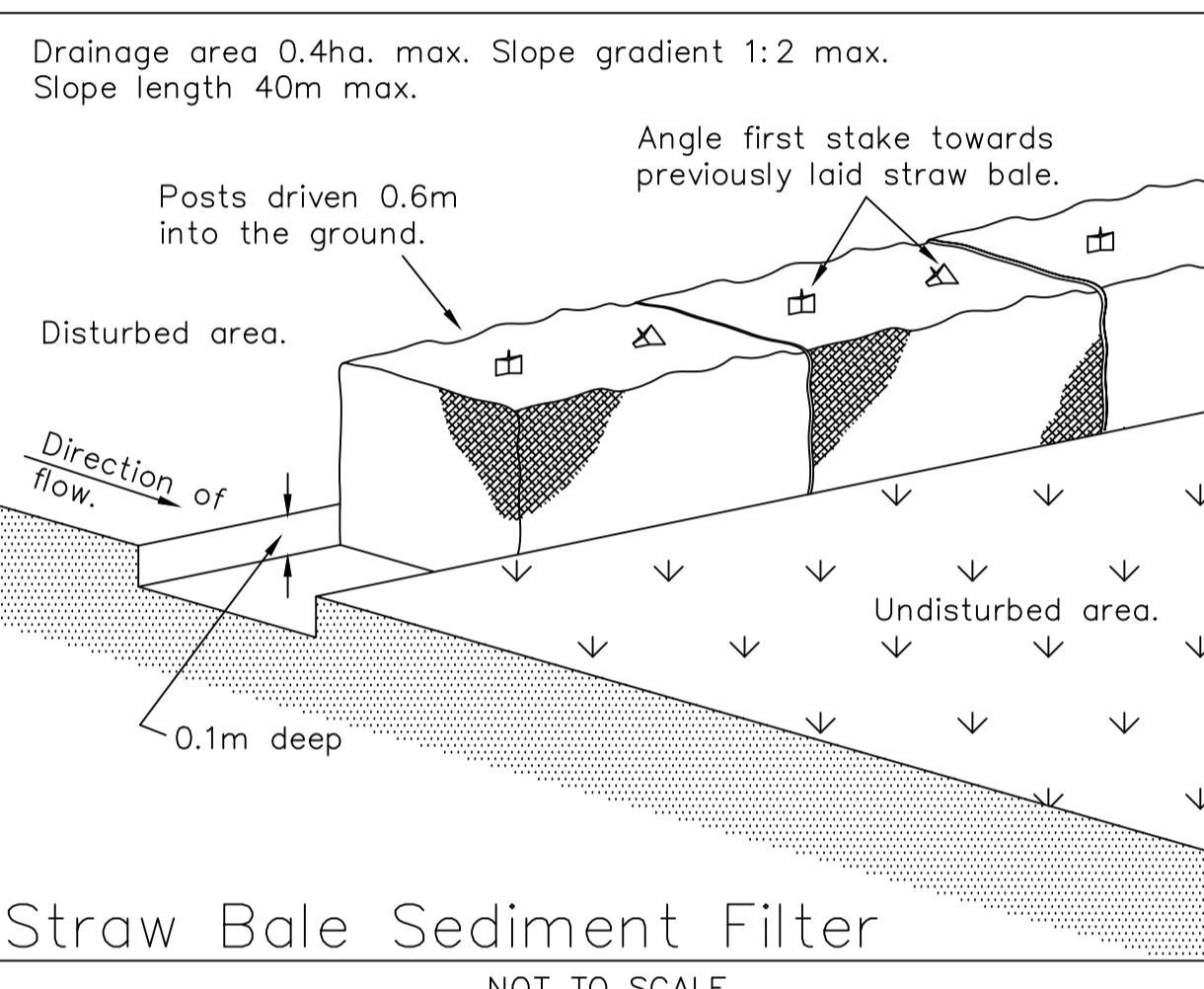
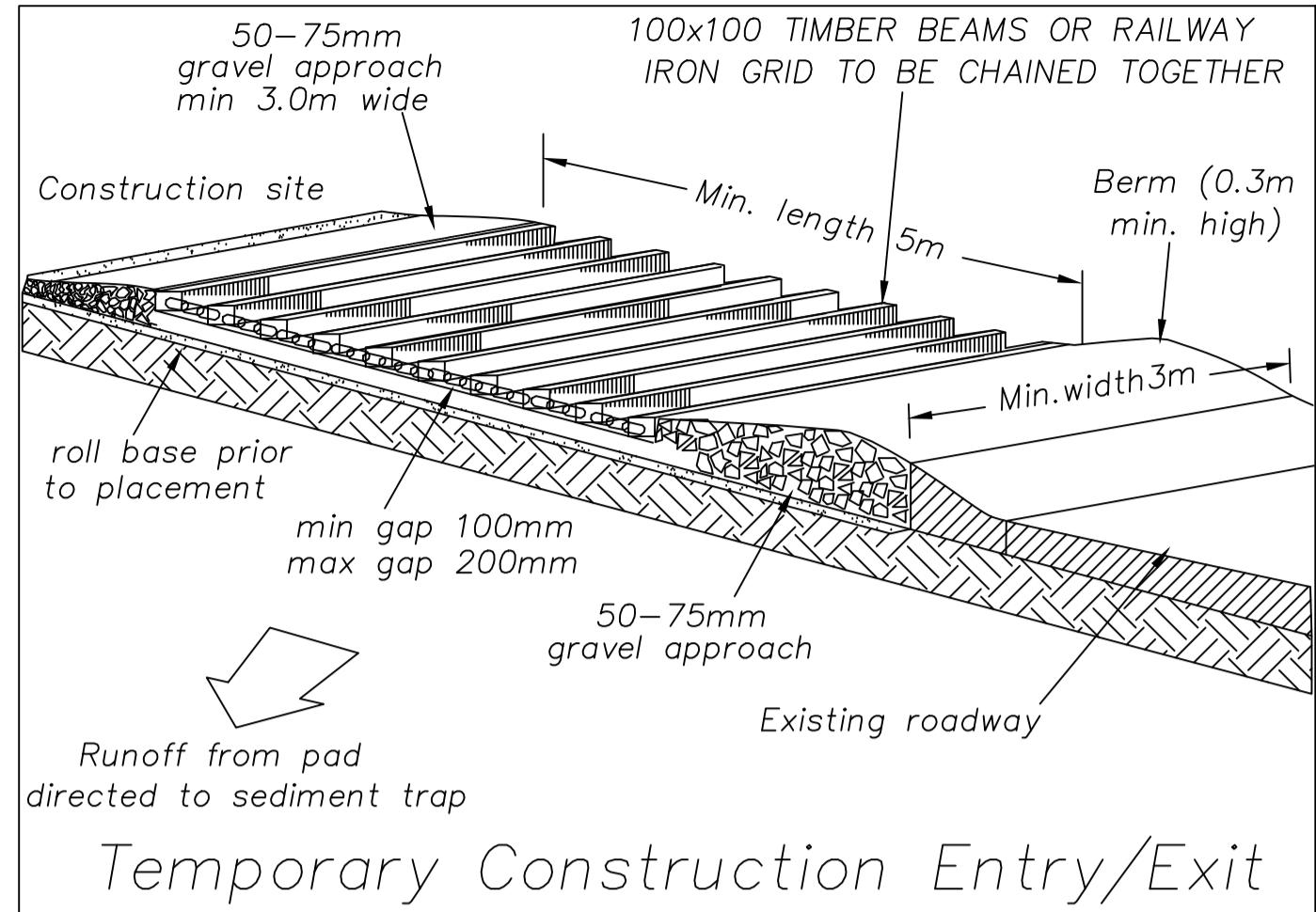
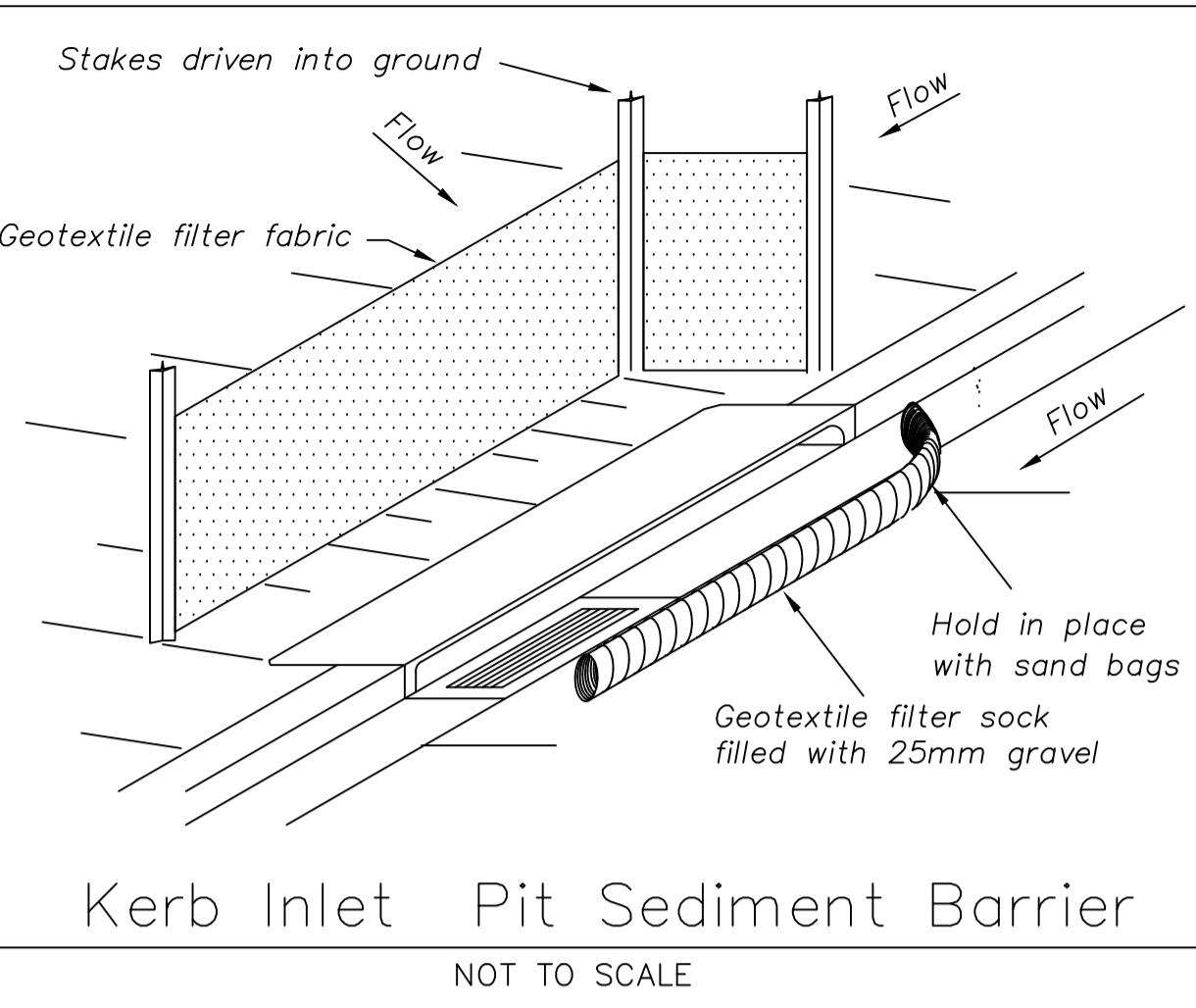
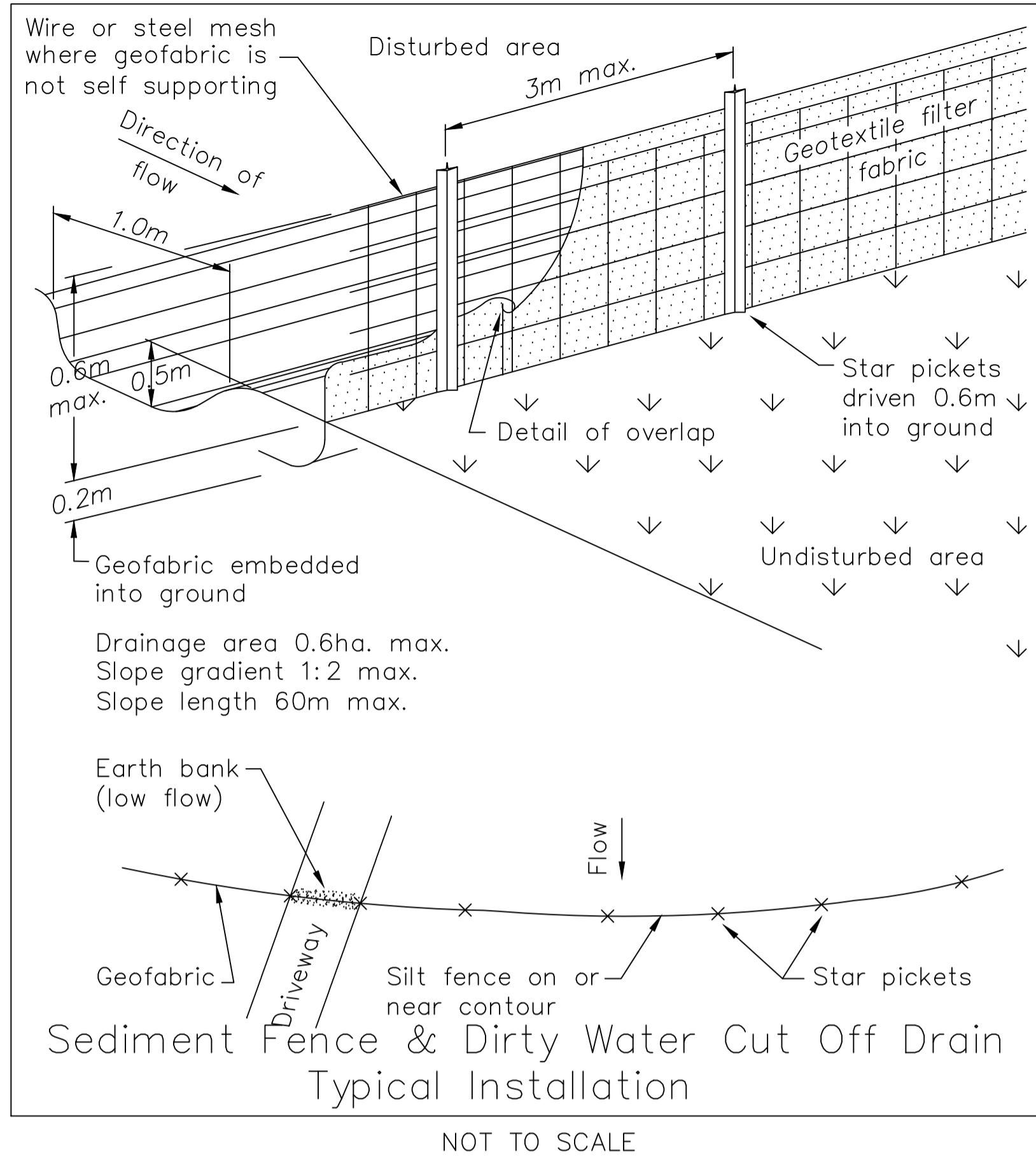
0 5 10 15 20 30 40 50
Meters

SCALE : - 1 : 500 @ A1
- 1 : 1000 @ A3

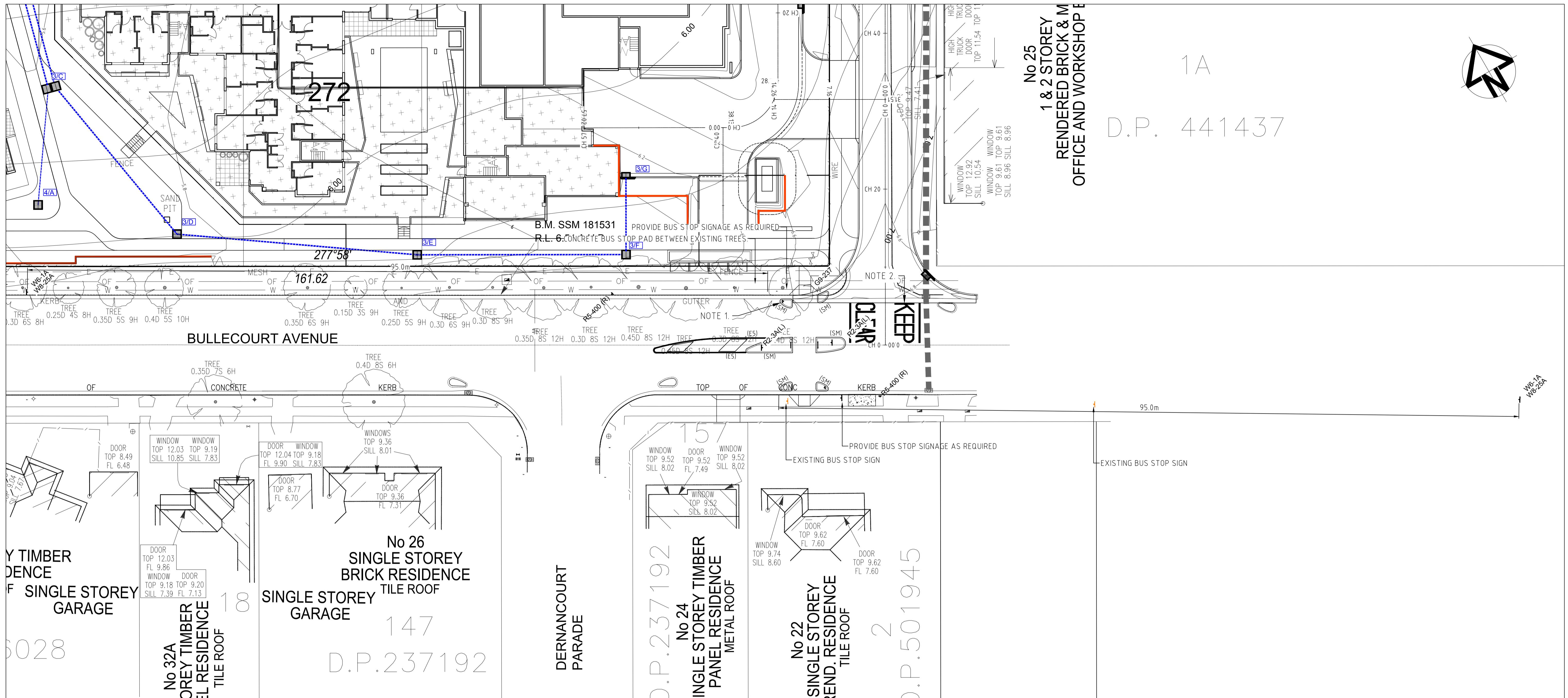
SOIL AND WATER MANAGEMENT PLAN



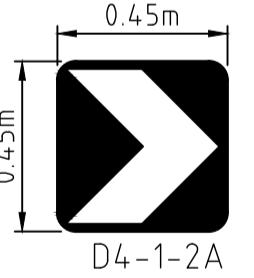
No.	DESCRIPTION	DRN	APP	DATE
C	REVISED ARCHITECTURAL PLANS	A.C.	A.C.	06.07.18
D	DRAFT DA ISSUE	A.C.	A.C.	16.12.18
E	REVISED TO SUIT CONSULTANTS COMMENTS	A.C.	A.C.	19.12.18
F	REVISED TO PLANNERS COMMENTS	A.C.	A.C.	25.01.19
G	REVISED TO COUNCIL COMMENTS	M.D.	a.c	16.08.19



No.	DESCRIPTION	DRN	APP	DATE
C	REVISED ARCHITECTURAL PLANS	A.C.	A.C.	06.07.18
D	DRAFT DA ISSUE	A.C.	A.C.	16.12.18
E	REVISED TO SUIT CONSULTANTS COMMENTS	A.C.	A.C.	19.12.18
F	REVISED TO PLANNERS COMMENTS	A.C.	A.C.	25.01.19
G	REVISED TO COUNCIL COMMENTS	M.D.	a.c	16.08.19



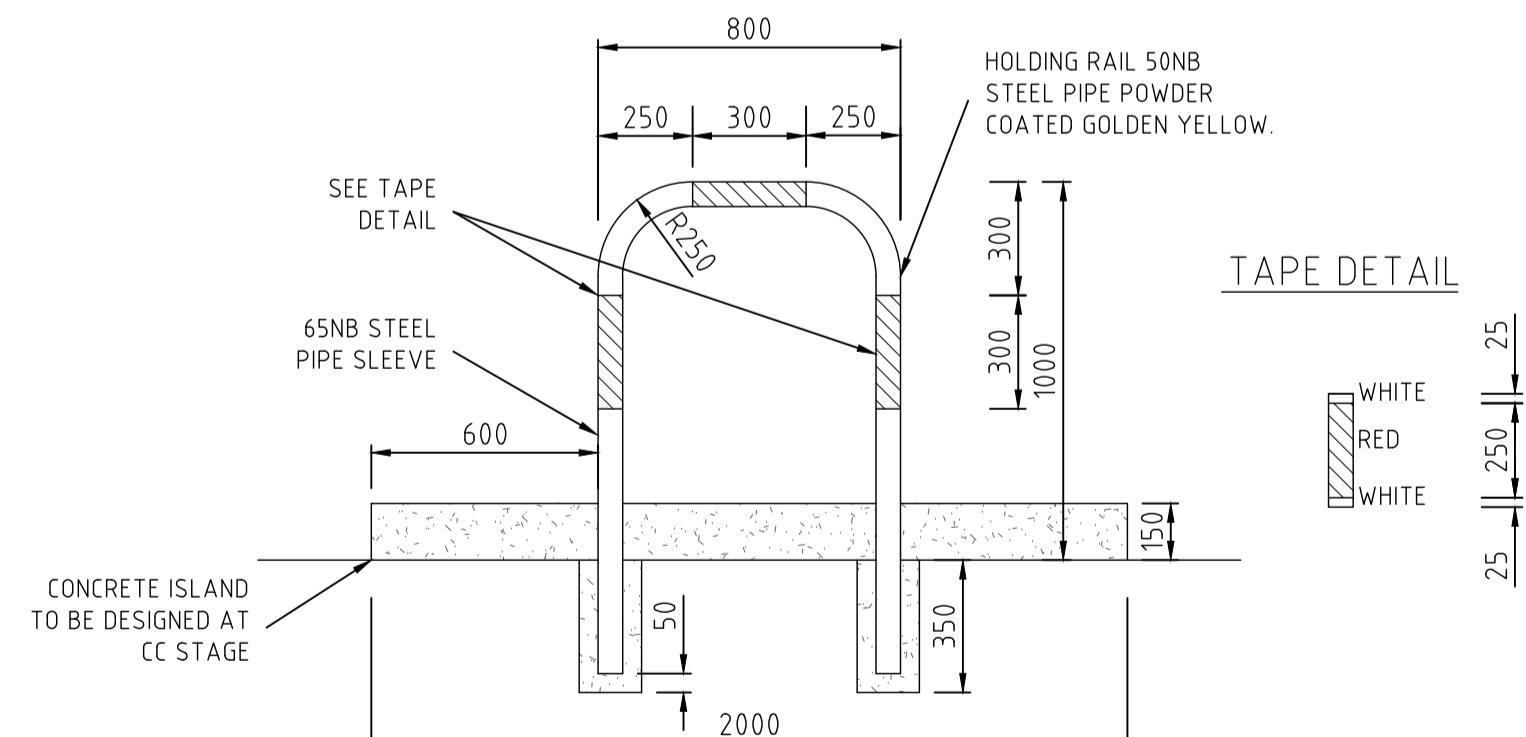
UNIDIRECTIONAL HAZARD MARKER
NOT TO SCALE



0.45m
0.45m
0.60m
KEEP LEFT
D4-1-2A
KEEP LEFT
NOT TO SCALE

0.45m
0.23m
0.45m
NO STOPPING
R5-400(L-R)
REFUGE ISLAND
W8-25A
NO STOPPING
NOT TO SCALE

**DO NOT
QUEUE
ACROSS
INTERSECTION**
G9-237
DO NOT QUEUE
NOT TO SCALE



ISLAND DETAIL
SCALE 1:20

NOTES:
1. CONSTRUCT PEDESTRIAN REFUGE IN ACCORDANCE WITH
RMS TECHNICAL DIRECTION TDT2011/01a.
2. "KEEP CLEAR" PAVEMENT MARKING ON THE EASTBOUND
LANE OF BULLECOURT AVENUE AS PER RMS DELINEATION
- SECTION 9.3.2.5 SPECIFICATIONS

0 10 20 30
Meters
SCALE : - 1: 250 @ A1
- 1: 500 @ A3

SIGNAGE PLAN

REVISIONS	Z.	DESCRIPTION
C		REVISED ARCHITECTURAL PLANS
D		DRAFT DA ISSUE
F		REVISED TO SUIT CONSULTANTS COMMENTS
M		REVISED TO PLANNERS COMMENTS
G		REVISED TO COUNCIL COMMENTS

Appendix B

Traffic Survey Results

Location	Bullecourt Lane	Duration	0630 - 0930
	Bullecourt Avenue		1530 - 1830
	Dernancourt Parade		-
	Bullecourt Avenue	Day/Date	Friday, 11 August 2017
Suburb	MILPERRA	Weather	-

All Vehicles Time Per 15 Mins	NORTH				EAST				SOUTH				WEST				TOTAL
	Bullecourt Lane				Bullecourt Avenue				Dernancourt Parade				Bullecourt Avenue				
	L	I	R	TOTAL	L	I	R	TOTAL	L	I	R	TOTAL	L	I	R	TOTAL	
6:30 - 6:45	0	0	0	0	1	36	0	37	1	0	1	2	0	87	2	89	128
6:45 - 7:00	0	0	0	0	1	32	0	33	2	0	2	4	0	133	2	135	172
7:00 - 7:15	0	0	0	0	6	45	1	52	1	0	1	2	0	111	0	111	165
7:15 - 7:30	0	0	0	0	3	59	0	62	0	0	1	1	0	152	0	152	215
7:30 - 7:45	0	0	0	0	2	55	0	57	1	0	1	2	0	147	2	149	208
7:45 - 8:00	0	0	0	0	8	48	0	56	4	0	0	4	0	155	2	157	217
8:00 - 8:15	0	0	0	0	4	56	0	60	3	0	0	3	0	189	4	193	256
8:15 - 8:30	0	0	0	0	1	66	0	67	8	0	1	9	0	173	4	177	253
8:30 - 8:45	0	0	0	0	3	64	0	67	6	0	0	6	0	154	1	155	228
8:45 - 9:00	0	0	0	0	3	72	0	75	1	0	1	2	0	171	6	177	254
9:00 - 9:15	0	0	1	1	5	59	0	64	3	0	3	6	0	154	4	158	229
9:15 - 9:30	0	0	0	0	8	72	1	81	0	0	2	2	0	135	3	138	221
Period End	0	0	1	1	45	664	2	711	30	0	13	43	0	1761	30	1791	2546
15:30 - 15:45	0	0	0	0	13	136	0	149	3	0	2	5	0	83	4	87	241
15:45 - 16:00	0	0	1	1	10	118	0	128	2	0	1	3	0	70	5	75	207
16:00 - 16:15	0	0	0	0	5	139	0	144	4	0	0	4	1	70	6	77	225
16:15 - 16:30	0	0	0	0	7	122	0	129	6	0	3	9	0	68	2	70	208
16:30 - 16:45	0	0	0	0	7	129	0	136	4	0	0	4	0	70	1	71	211
16:45 - 17:00	0	0	0	0	5	131	0	136	2	0	1	3	0	60	2	62	201
17:00 - 17:15	0	0	0	0	7	166	0	173	2	0	3	5	0	38	2	40	218
17:15 - 17:30	0	0	1	1	15	158	0	173	3	0	1	4	0	40	1	41	219
17:30 - 17:45	0	0	0	0	6	117	0	123	1	0	1	2	0	63	2	65	190
17:45 - 18:00	1	0	0	1	6	11	0	17	3	0	2	5	0	69	0	69	92
18:00 - 18:15	0	0	0	0	4	117	0	121	1	0	4	5	0	54	3	57	183
18:15 - 18:30	0	0	0	0	4	91	0	95	3	0	4	7	0	45	0	45	147
Period End	1	0	2	3	89	1435	0	1524	34	0	22	56	1	730	28	759	2342

Traffic Information Specialists

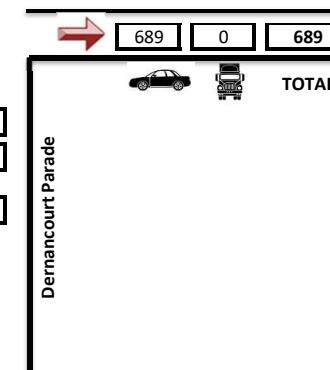
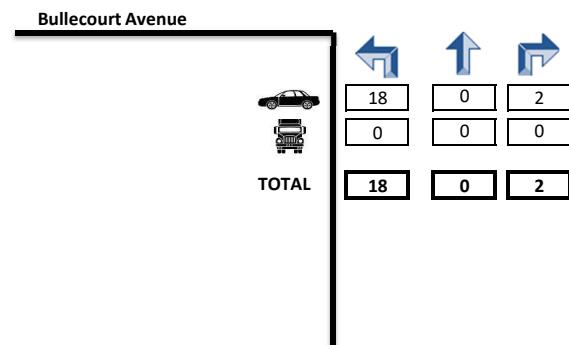
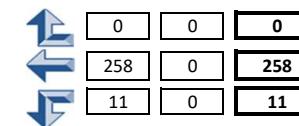
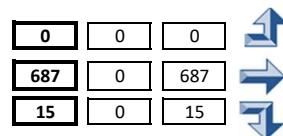
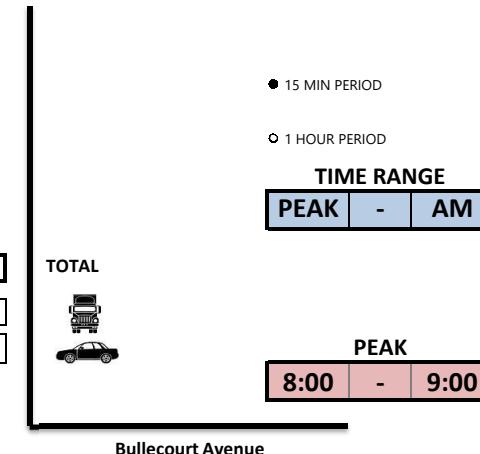
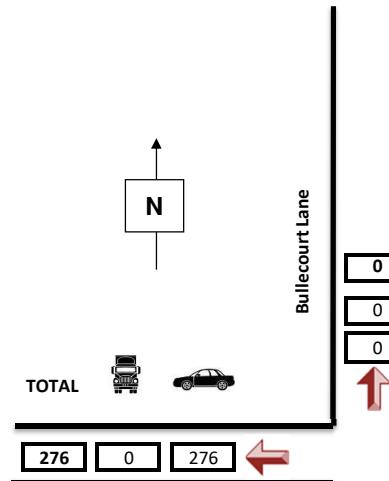
ABN: 42 613 389 923

Email info@trafficinfospecialist.com.au

All Vehicles		NORTH Bullecourt Lane				EAST Bullecourt Avenue				SOUTH Dernancourt Parade				WEST Bullecourt Avenue				TOTAL
Time Per Hour		L	I	R	TOTAL	L	I	R	TOTAL	L	I	R	TOTAL	L	I	R	TOTAL	
6:30 - 7:30		0	0	0	0	11	172	1	184	4	0	5	9	0	483	4	487	680
6:45 - 7:45		0	0	0	0	12	191	1	204	4	0	5	9	0	543	4	547	760
7:00 - 8:00		0	0	0	0	19	207	1	227	6	0	3	9	0	565	4	569	805
7:15 - 8:15		0	0	0	0	17	218	0	235	8	0	2	10	0	643	8	651	896
7:30 - 8:30		0	0	0	0	15	225	0	240	16	0	2	18	0	664	12	676	934
7:45 - 8:45		0	0	0	0	16	234	0	250	21	0	1	22	0	671	11	682	954
8:00 - 9:00		0	0	0	0	11	258	0	269	18	0	2	20	0	687	15	702	991
8:15 - 9:15		0	0	1	1	12	261	0	273	18	0	5	23	0	652	15	667	964
8:30 - 9:30		0	0	1	1	19	267	1	287	10	0	6	16	0	614	14	628	932
Period End		0	0	2	2	132	2033	4	2169	105	0	31	136	0	5522	87	5609	7916
15:30 - 16:30		0	0	1	1	35	515	0	550	15	0	6	21	1	291	17	309	881
15:45 - 16:45		0	0	1	1	29	508	0	537	16	0	4	20	1	278	14	293	851
16:00 - 17:00		0	0	0	0	24	521	0	545	16	0	4	20	1	268	11	280	845
16:15 - 17:15		0	0	0	0	26	548	0	574	14	0	7	21	0	236	7	243	838
16:30 - 17:30		0	0	1	1	34	584	0	618	11	0	5	16	0	208	6	214	849
16:45 - 17:45		0	0	1	1	33	572	0	605	8	0	6	14	0	201	7	208	828
17:00 - 18:00		1	0	1	2	34	452	0	486	9	0	7	16	0	210	5	215	719
17:15 - 18:15		1	0	1	2	31	403	0	434	8	0	8	16	0	226	6	232	684
17:30 - 18:30		1	0	0	1	20	336	0	356	8	0	11	19	0	231	5	236	612
Period End		3	0	6	9	266	4439	0	4705	105	0	58	163	3	2149	78	2230	7107

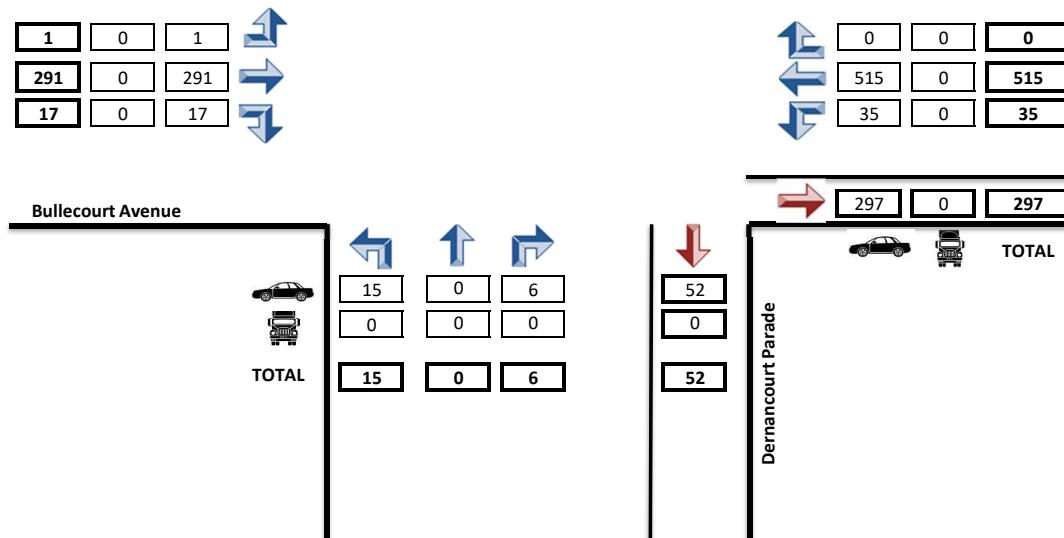
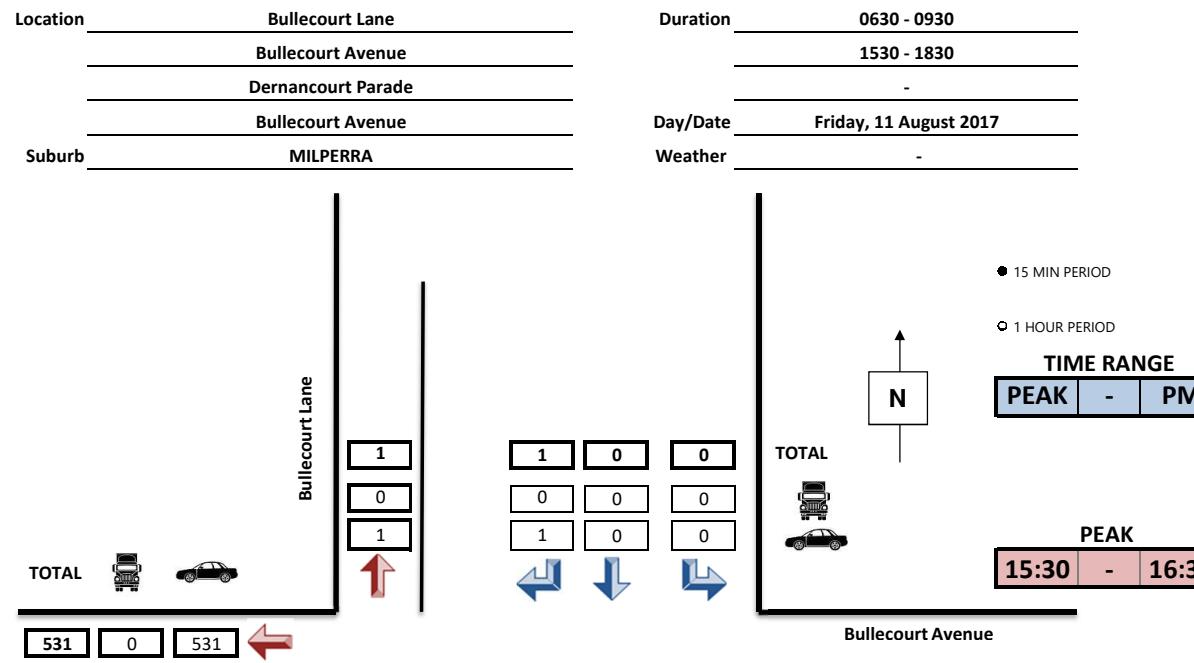
Location Bullecourt Lane
 Bullecourt Avenue
 Dernancourt Parade
 Bullecourt Avenue
 Suburb MILPERRA

Duration 0630 - 0930
 1530 - 1830
 -
 Day/Date Friday, 11 August 2017
 Weather -



Traffic Information Specialists

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Traffic Information Specialists

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Appendix C

SIDRA Model Results

MOVEMENT SUMMARY

▽ Site: 101 [EX AM BULLECOURT LANE - BULLECOURT]

⊕ Network: N101 [EX AM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows		Arrival Flows		Deg. Satn	Average Delay	Level of Service	Aver. Back of Queue Vehicles	Prop. Queued	Effective Stop Rate	Aver. Cycles	Avg. Speed	
		Total	HV	Total	HV	v/c	sec		veh	m			km/h	
East: BULLECOURT														
5	T1	292	7.0	292	7.0	0.158	0.0	LOS A	0.0	0.1	0.01	0.00	0.01	58.2
6	R2	1	7.0	1	7.0	0.158	8.2	LOS A	0.0	0.1	0.01	0.00	0.01	47.3
Approach		293	7.0	293	7.0	0.158	0.1	NA	0.0	0.1	0.01	0.00	0.01	58.0
North: BULLECOURT LANE														
7	L2	1	7.0	1	7.0	0.005	9.6	LOS A	0.0	0.0	0.68	0.74	0.68	27.5
9	R2	1	7.0	1	7.0	0.005	14.0	LOS B	0.0	0.0	0.68	0.74	0.68	27.3
Approach		2	7.0	2	7.0	0.005	11.8	LOS B	0.0	0.0	0.68	0.74	0.68	27.4
West: BULLECOURT														
10	L2	1	7.0	1	7.0	0.401	5.7	LOS A	0.0	0.0	0.00	0.00	0.00	55.7
11	T1	747	7.0	747	7.0	0.401	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Approach		748	7.0	748	7.0	0.401	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
All Vehicles		1043	7.0	1043	7.0	0.401	0.1	NA	0.0	0.1	0.00	0.00	0.00	59.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: TRANSPORT AND TRAFFIC PLANNING ASSOCIATES | Processed: Tuesday, January 22, 2019 5:56:33 PM

Project: T:\WORK17\17161 - ANGLICARE, BULLECOURT AVE, MILPERRA\MODELLING\Bullecourt.sip8

MOVEMENT SUMMARY

▼ Site: 101 [EX AM DERNANCOURT - BULLECOURT]

◆◆ Network: N101 [EX AM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows		Arrival Flows		Deg. Satn	Average Delay	Level of Service	Aver. Back of Queue Vehicles	Prop. Queued	Effective Stop Rate	Aver. Cycles	Avg. Speed	
		Total	HV	Total	HV	v/c	sec		veh	m			km/h	
South: DERNANCOURT														
1	L2	20	7.0	20	7.0	0.016	6.6	LOS A	0.0	0.2	0.35	0.57	0.35	51.1
3	R2	2	7.0	2	7.0	0.007	14.1	LOS B	0.0	0.1	0.74	0.82	0.74	41.1
Approach		22	7.0	22	7.0	0.016	7.3	LOS A	0.0	0.2	0.39	0.59	0.39	50.3
East: BULLECOURT														
4	L2	12	7.0	12	7.0	0.157	3.1	LOS A	0.0	0.0	0.00	0.02	0.00	56.2
5	T1	280	7.0	280	7.0	0.157	0.0	LOS A	0.0	0.0	0.00	0.02	0.00	59.6
Approach		292	7.0	292	7.0	0.157	0.1	NA	0.0	0.0	0.00	0.02	0.00	59.4
West: BULLECOURT														
11	T1	745	7.0	745	7.0	0.412	0.1	LOS A	0.1	0.7	0.03	0.01	0.03	59.3
12	R2	16	7.0	16	7.0	0.412	7.5	LOS A	0.1	0.7	0.03	0.01	0.03	57.1
Approach		761	7.0	761	7.0	0.412	0.2	NA	0.1	0.7	0.03	0.01	0.03	59.2
All Vehicles		1074	7.0	1074	7.0	0.412	0.3	NA	0.1	0.7	0.03	0.03	0.03	58.8

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: TRANSPORT AND TRAFFIC PLANNING ASSOCIATES | Processed: Tuesday, January 22, 2019 5:56:33 PM

Project: T:\WORK17\17161 - ANGLICARE, BULLECOURT AVE, MILPERRA\MODELLING\Bullecourt.sip8

MOVEMENT SUMMARY

▽ Site: 101 [EX PM BULLECOURT LANE - BULLECOURT]

◆◆ Network: N101 [EX PM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows			Arrival Flows			Deg. Satn	Average Delay v/c	Level of Service	Aver. Back of Queue Vehicles	Prop. Queued	Effective Stop Rate	Aver. Average Cycles Speed km/h
		Total veh/h	HV %	Total veh/h	HV %			sec		veh	m			
East: BULLECOURT														
5	T1	596	7.0	596	7.0	0.321	0.0	LOS A	0.0	0.0	0.00	0.00	0.00 59.7	
6	R2	1	7.0	1	7.0	0.321	4.7	LOS A	0.0	0.0	0.00	0.00	0.00 47.5	
Approach		597	7.0	597	7.0	0.321	0.0	NA	0.0	0.0	0.00	0.00	0.00 59.5	
North: BULLECOURT LANE														
7	L2	1	7.0	1	7.0	0.004	6.7	LOS A	0.0	0.0	0.48	0.62	0.48 30.7	
9	R2	1	7.0	1	7.0	0.004	11.7	LOS B	0.0	0.0	0.48	0.62	0.48 31.0	
Approach		2	7.0	2	7.0	0.004	9.2	LOS A	0.0	0.0	0.48	0.62	0.48 30.8	
West: BULLECOURT														
10	L2	1	7.0	1	7.0	0.173	5.6	LOS A	0.0	0.0	0.00	0.00	0.00 55.8	
11	T1	322	7.0	322	7.0	0.173	0.0	LOS A	0.0	0.0	0.00	0.00	0.00 59.9	
Approach		323	7.0	323	7.0	0.173	0.0	NA	0.0	0.0	0.00	0.00	0.00 59.9	
All Vehicles		923	7.0	923	7.0	0.321	0.0	NA	0.0	0.0	0.00	0.00	0.00 59.7	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

▽ Site: 101 [EX PM DERNANCOURT - BULLECOURT]

◆◆ Network: N101 [EX PM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows		Arrival Flows		Deg. Satn	Average Delay v/c	Level of Service sec	Aver. Back of Queue		Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
		Total veh/h	HV %	Total veh/h	HV %				Vehicles veh	Distance m				
South: DERNANCOURT														
1	L2	16	7.0	16	7.0	0.019	8.1	LOS A	0.0	0.2	0.51	0.67	0.51	50.2
3	R2	7	7.0	7	7.0	0.015	11.7	LOS B	0.0	0.1	0.66	0.81	0.66	43.5
Approach		23	7.0	23	7.0	0.019	9.2	LOS A	0.0	0.2	0.55	0.71	0.55	48.8
East: BULLECOURT														
4	L2	38	7.0	38	7.0	0.321	3.1	LOS A	0.0	0.0	0.00	0.04	0.00	56.0
5	T1	558	7.0	558	7.0	0.321	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.4
Approach		596	7.0	596	7.0	0.321	0.2	NA	0.0	0.0	0.00	0.04	0.00	59.1
West: BULLECOURT														
11	T1	317	7.0	317	7.0	0.191	0.4	LOS A	0.1	0.8	0.10	0.04	0.10	57.7
12	R2	18	7.0	18	7.0	0.191	9.2	LOS A	0.1	0.8	0.10	0.04	0.10	56.4
Approach		335	7.0	335	7.0	0.191	0.9	NA	0.1	0.8	0.10	0.04	0.10	57.6
All Vehicles		954	7.0	954	7.0	0.321	0.6	NA	0.1	0.8	0.05	0.05	0.05	58.0

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

▽ Site: 101 [POST-DEV AM BULLECOURT LANE - BULLECOURT]

⊕ Network: N101 [POST-DEV AM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows			Arrival Flows		Deg. Satn	Average Delay v/c	Level of Service sec	Aver. Back of Queue Vehicles	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Aver. Average Speed km/h
		Total veh/h	HV %	Total veh/h	HV %				veh	Distance m				
East: BULLECOURT														
5	T1	378	6.9	378	6.9	0.235	1.1	LOS A	0.2	1.6	0.13	0.02	0.15	36.9
6	R2	15	0.6	15	0.6	0.235	12.4	LOS B	0.2	1.6	0.13	0.02	0.15	44.6
Approach		393	6.7	393	6.7	0.235	1.5	NA	0.2	1.6	0.13	0.02	0.15	38.0
North: BULLECOURT LANE														
7	L2	26	0.4	26	0.4	0.092	12.6	LOS B	0.1	0.8	0.76	0.90	0.76	26.1
9	R2	11	0.9	11	0.9	0.092	14.8	LOS B	0.1	0.8	0.76	0.90	0.76	25.6
Approach		38	0.5	38	0.5	0.092	13.3	LOS B	0.1	0.8	0.76	0.90	0.76	26.0
West: BULLECOURT														
10	L2	6	1.8	6	1.8	0.518	5.6	LOS A	0.0	0.0	0.00	0.00	0.00	56.4
11	T1	962	7.0	962	7.0	0.518	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
Approach		967	6.9	967	6.9	0.518	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.7
All Vehicles		1398	6.7	1398	6.7	0.518	0.8	NA	0.2	1.6	0.06	0.03	0.06	57.0

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

▽ Site: 101 [POST-DEV AM DERNANCOURT - BULLECOURT] ⚡ Network: N101 [POST-DEV AM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows		Arrival Flows		Deg. Satn	Average Delay v/c	Level of Service sec	Aver. Back of Queue Vehicles	Prop. Queued veh	Effective Stop Rate m	Aver. No. Cycles	Aver. Speed km/h
		Total veh/h	HV %	Total veh/h	HV %								
South: DERNANCOURT													
1	L2	25	7.0	25	7.0	0.023	7.0	LOS A	0.0	0.3	0.41	0.61	0.41
3	R2	4	4.7	4	4.7	0.013	15.0	LOS B	0.0	0.1	0.80	0.92	0.80
Approach		29	6.7	29	6.7	0.023	8.2	LOS A	0.0	0.3	0.47	0.65	0.47
East: BULLECOURT													
4	L2	17	6.4	17	6.4	0.209	3.1	LOS A	0.0	0.0	0.00	0.02	0.00
5	T1	372	6.7	372	6.7	0.209	0.0	LOS A	0.0	0.0	0.00	0.02	0.00
Approach		389	6.7	389	6.7	0.209	0.1	NA	0.0	0.0	0.00	0.02	0.00
West: BULLECOURT													
11	T1	959	7.0	959	7.0	0.533	0.2	LOS A	0.2	1.5	0.04	0.01	0.06
12	R2	21	7.0	21	7.0	0.533	9.1	LOS A	0.2	1.5	0.04	0.01	0.06
Approach		980	7.0	980	7.0	0.533	0.4	NA	0.2	1.5	0.04	0.01	0.06
All Vehicles		1398	6.9	1398	6.9	0.533	0.5	NA	0.2	1.5	0.04	0.03	0.05
58.6													

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

▽ Site: 101 [POST-DEV PM BULLECOURT LANE - BULLECOURT]

⊕ Network: N101 [POST-DEV PM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows			Arrival Flows		Deg. Satn	Average Delay v/c	Level of Service sec	Aver. Back of Queue Vehicles	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Aver. Average Speed km/h
		Total veh/h	HV %	Total veh/h	HV %				veh	Distance m				
East: BULLECOURT														
5	T1	848	7.0	848	7.0	0.473	0.2	LOS A	0.2	1.2	0.05	0.01	0.06	52.8
6	R2	19	0.6	19	0.6	0.473	6.6	LOS A	0.2	1.2	0.05	0.01	0.06	48.3
Approach		868	6.8	868	6.8	0.473	0.3	NA	0.2	1.2	0.05	0.01	0.06	52.4
North: BULLECOURT LANE														
7	L2	12	0.9	12	0.9	0.025	7.3	LOS A	0.0	0.2	0.51	0.69	0.51	30.9
9	R2	5	2.3	5	2.3	0.025	13.9	LOS B	0.0	0.2	0.51	0.69	0.51	31.2
Approach		17	1.3	17	1.3	0.025	9.1	LOS A	0.0	0.2	0.51	0.69	0.51	30.9
West: BULLECOURT														
10	L2	10	1.1	10	1.1	0.252	5.6	LOS A	0.0	0.0	0.00	0.01	0.00	56.4
11	T1	460	7.0	460	7.0	0.252	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.6
Approach		470	6.8	470	6.8	0.252	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.5
All Vehicles		1354	6.8	1354	6.8	0.473	0.4	NA	0.2	1.2	0.04	0.02	0.05	57.5

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

▽ Site: 101 [POST-DEV PM DERNANCOURT - BULLECOURT] ⚡ Network: N101 [POST-DEV PM PEAK]

New Site

Site Category: (None)

Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows		Arrival Flows		Deg. Satn	Average Delay v/c	Level of Service sec	Aver. Back of Queue Vehicles	Prop. Queued veh	Effective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
		Total veh/h	HV %	Total veh/h	HV %				Distance m				
South: DERNANCOURT													
1	L2	23	7.0	23	7.0	0.040	10.5	LOS B	0.1	0.4	0.65	0.82	0.65 48.3
3	R2	10	6.3	10	6.3	0.029	13.8	LOS B	0.0	0.2	0.77	0.91	0.77 41.4
Approach		33	6.8	33	6.8	0.040	11.5	LOS B	0.1	0.4	0.68	0.85	0.68 46.7
East: BULLECOURT													
4	L2	55	6.9	55	6.9	0.460	3.1	LOS A	0.0	0.0	0.00	0.04	0.00 56.0
5	T1	801	6.9	801	6.9	0.460	0.0	LOS A	0.0	0.0	0.00	0.04	0.00 59.4
Approach		856	6.9	856	6.9	0.460	0.2	NA	0.0	0.0	0.00	0.04	0.00 59.1
West: BULLECOURT													
11	T1	455	6.9	455	6.9	0.292	1.2	LOS A	0.3	2.5	0.17	0.04	0.21 55.2
12	R2	26	7.0	26	7.0	0.292	13.9	LOS B	0.3	2.5	0.17	0.04	0.21 55.4
Approach		481	6.9	481	6.9	0.292	1.9	NA	0.3	2.5	0.17	0.04	0.21 55.3
All Vehicles		1370	6.9	1370	6.9	0.460	1.1	NA	0.3	2.5	0.08	0.06	0.09 57.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

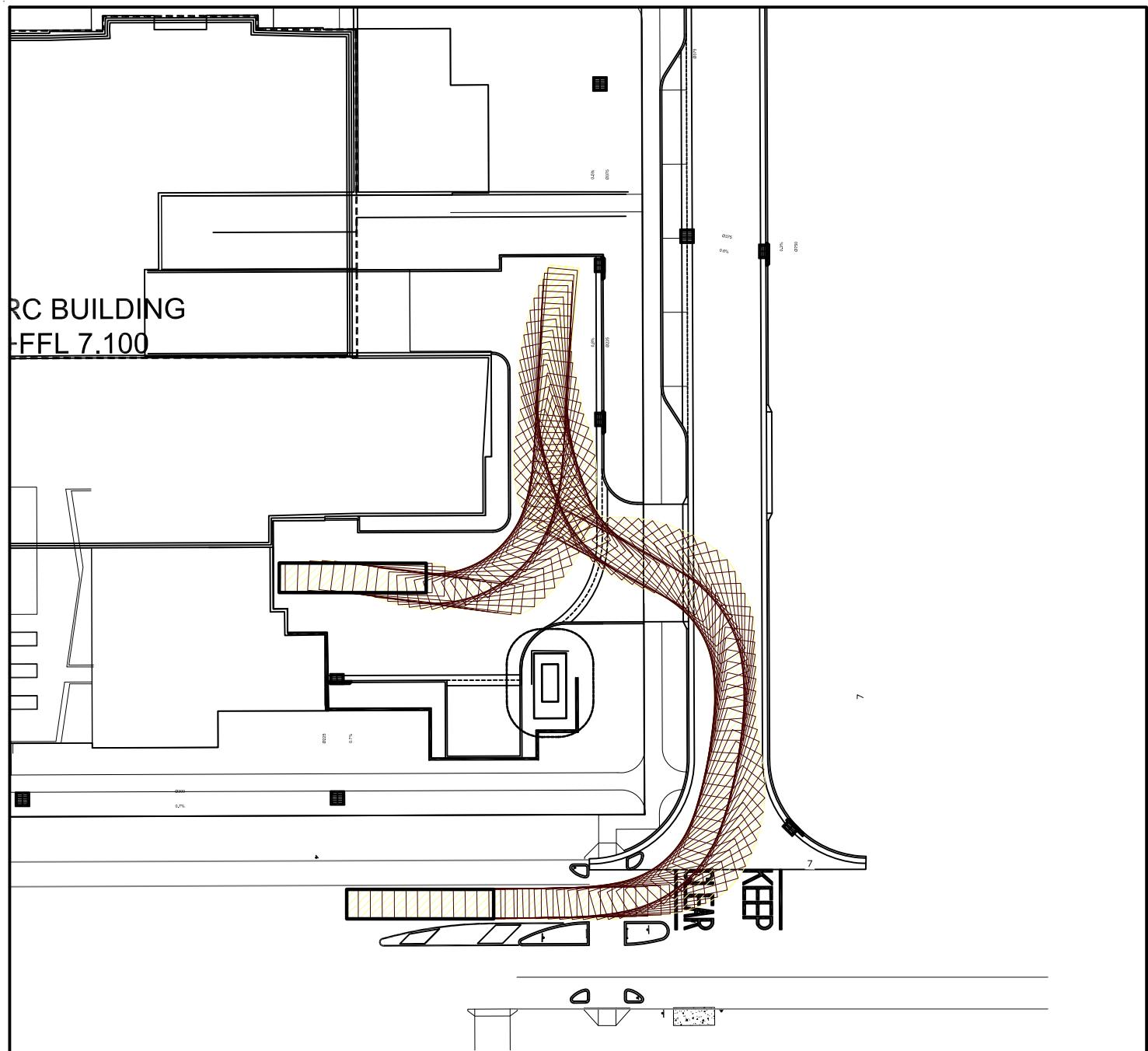
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Appendix D

Turning Path Assessment



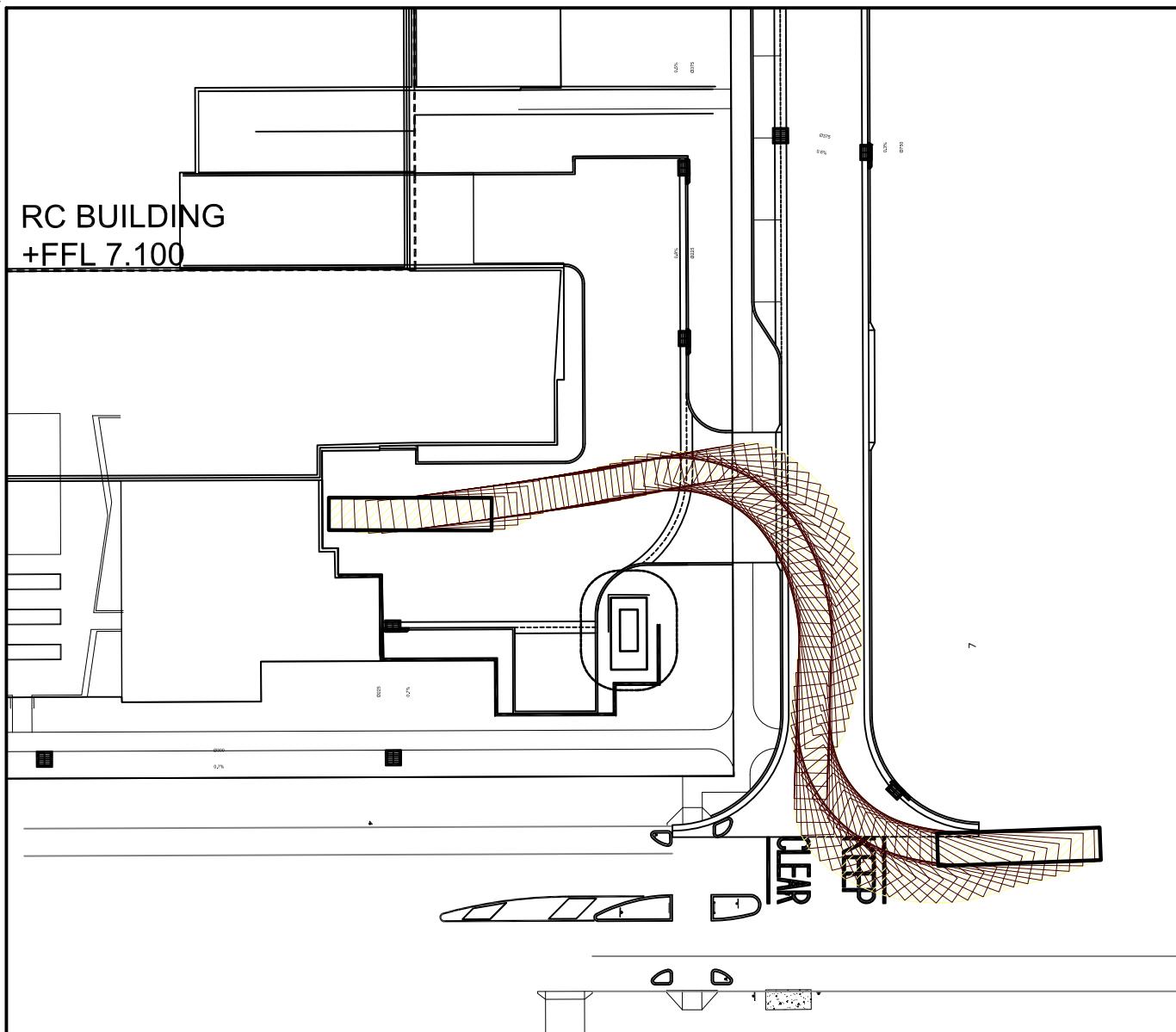
LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V5.00a in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS
OF A 12.5m RIGID
VEHICLE ENTERING THE SITE**

RC BUILDING
+FFL 7.100



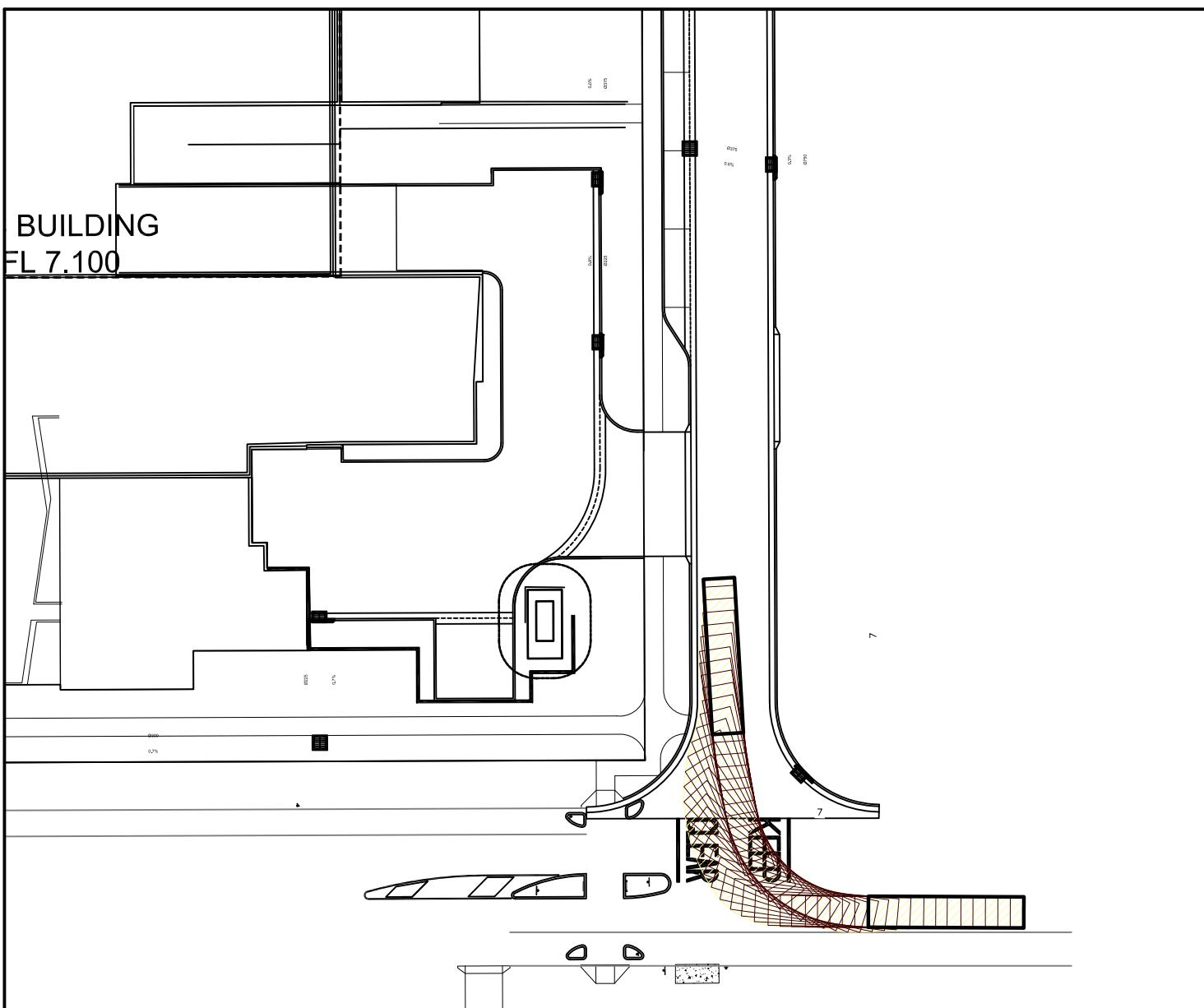
LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V5.00a in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS
OF A 12.5m RIGID
VEHICLE EXITING THE SITE**

SP 2



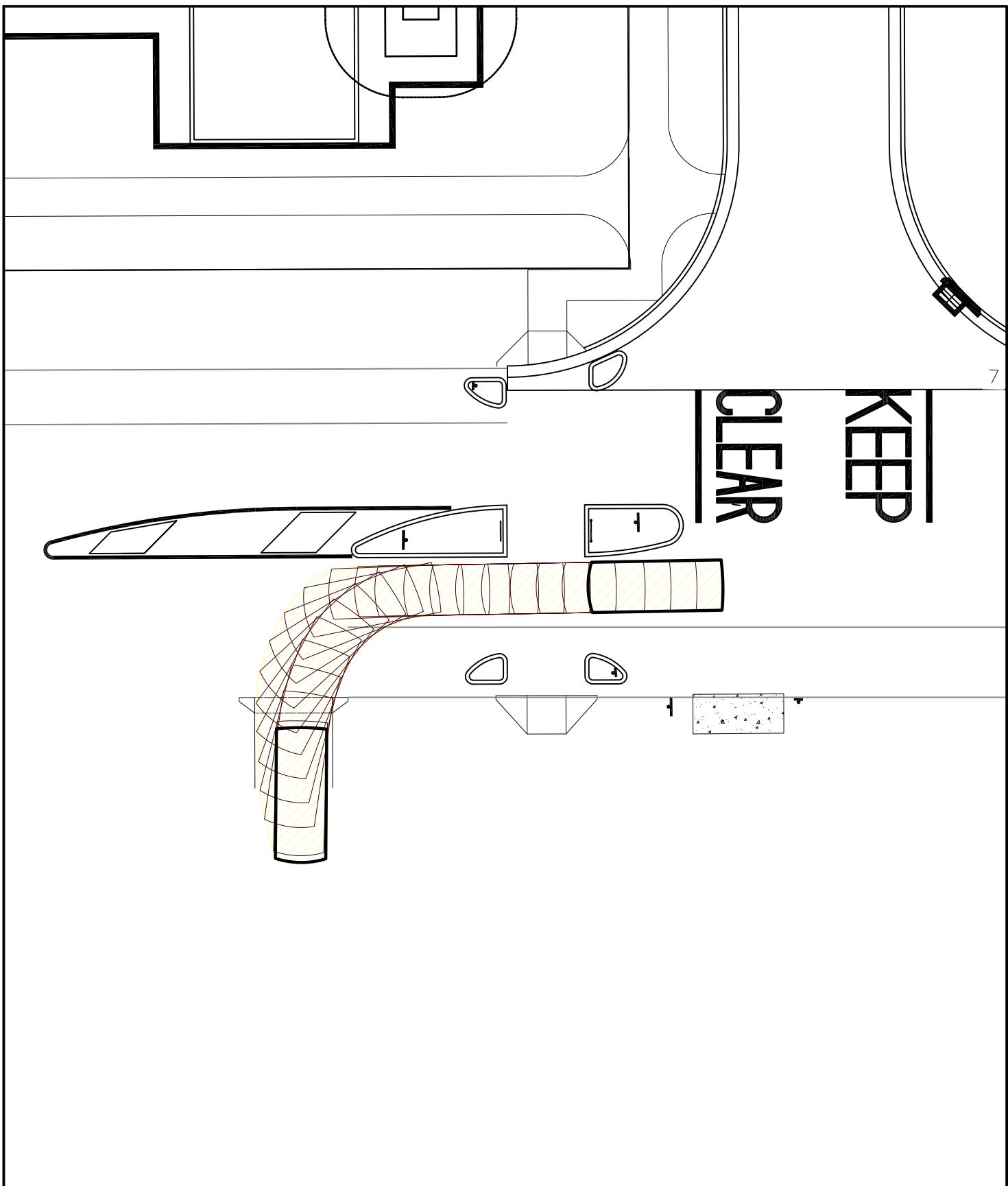
LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V5.00a in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS
OF A 12.5m RIGID
VEHICLE ENTERING THE SITE**

SP 3



LEGEND

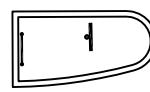
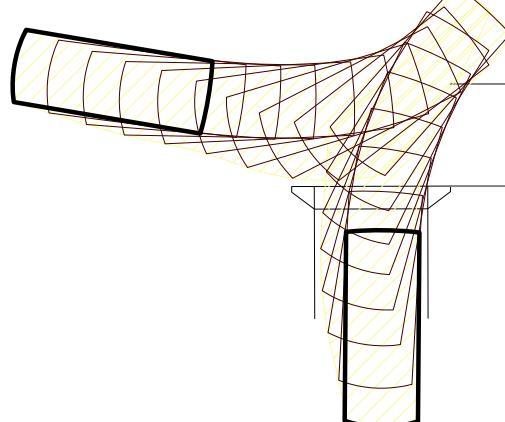
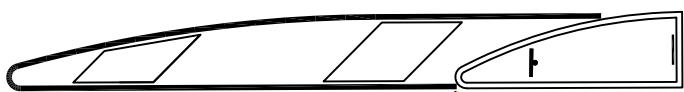
This drawing has been prepared using vehicle modelling computer software AutoTrack V5.00a in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



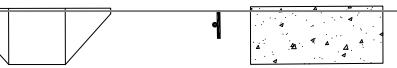
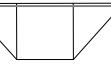
**SWEPT PATH ANALYSIS
OF A 99th PERCENTILE
VEHICLE ENTERING THE
DRIVEWAY OPPOSITE SITE**

0225

0.7%



CLEAR



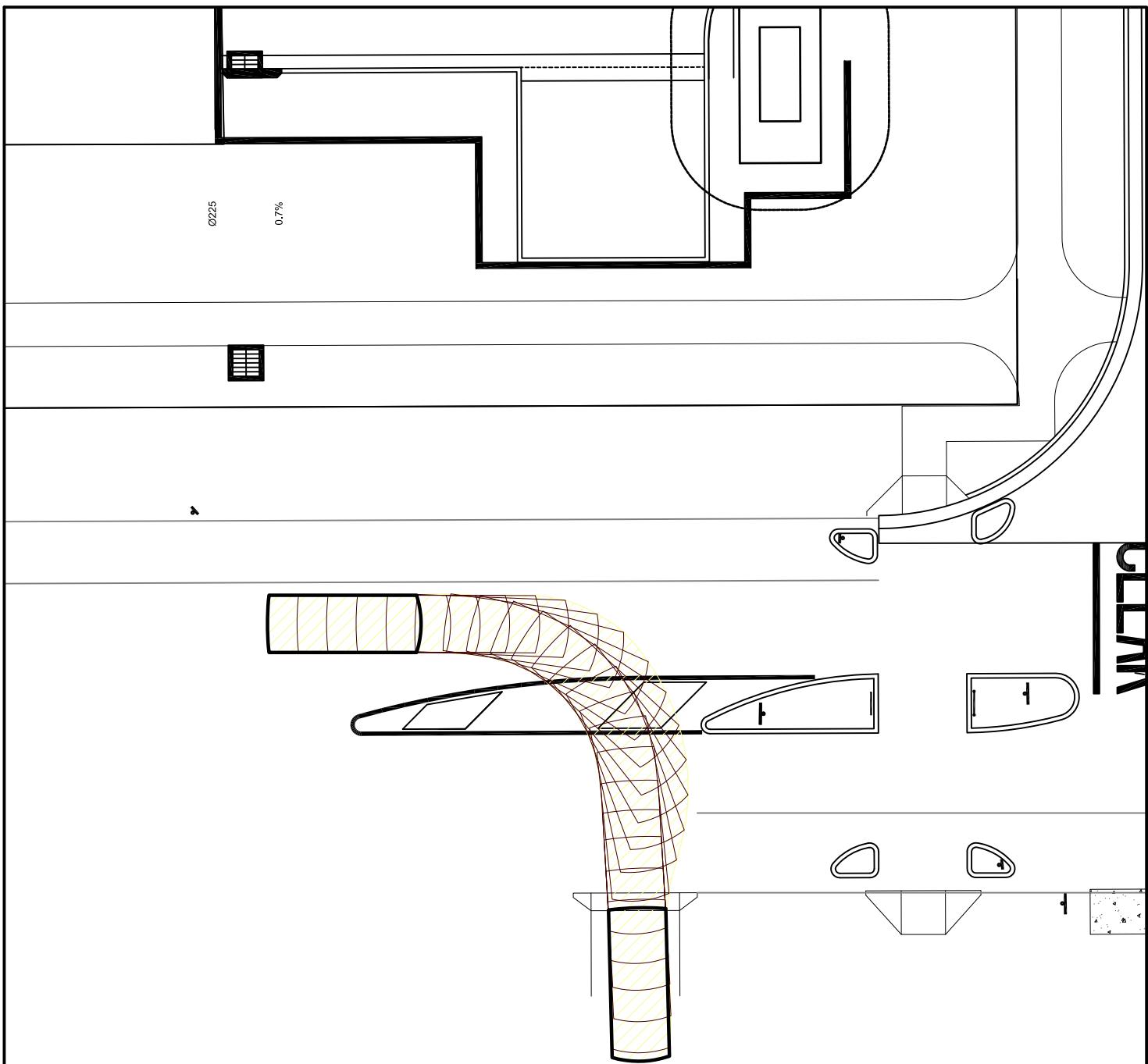
LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V5.00a in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS
OF A 99th PERCENTILE
VEHICLE EXITING THE
DRIVEWAY OPPOSITE SITE**

SP 5



LEGEND

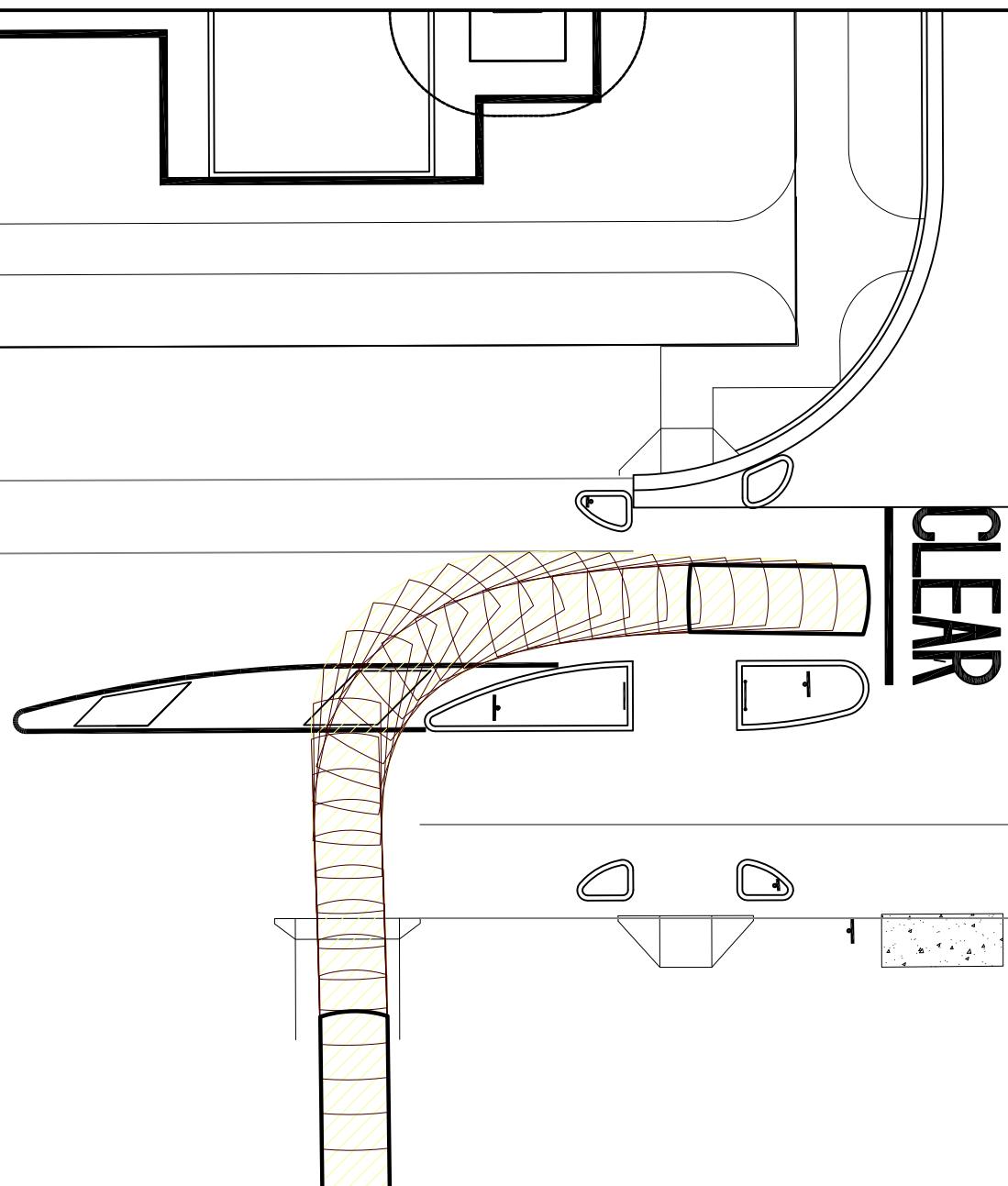
This drawing has been prepared using vehicle modelling computer software AutoTrack V5.00a in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS
OF A 99th PERCENTILE
VEHICLE ENTERING THE
DRIVEWAY OPPOSITE SITE**

Ø225

0.7%



LEGEND

This drawing has been prepared using vehicle modelling computer software AutoTrack V5.00a in conjunction with AutoCAD 2013. The vehicle used is based upon vehicle data provided by Austroads and incorporates a reasonable degree of tolerance. However, it is not possible to account for all vehicle types/characteristics and/or driver ability.



**SWEPT PATH ANALYSIS
OF A 99th PERCENTILE
VEHICLE EXITING THE
DRIVEWAY OPPOSITE SITE**

Transport and Traffic Planning Associates